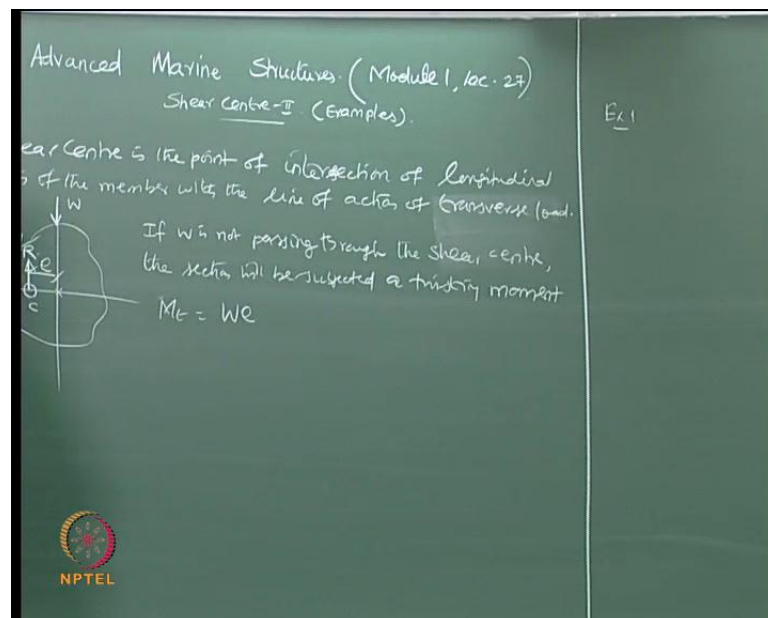


Advanced Marine Structures
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Lecture - 27
Shear centre – II – Examples

So, we will continue the lectures on advanced marine structures. We will have the lecture 27 on module 1. We have already discussed about the necessity for shear centre, study of shear centre.

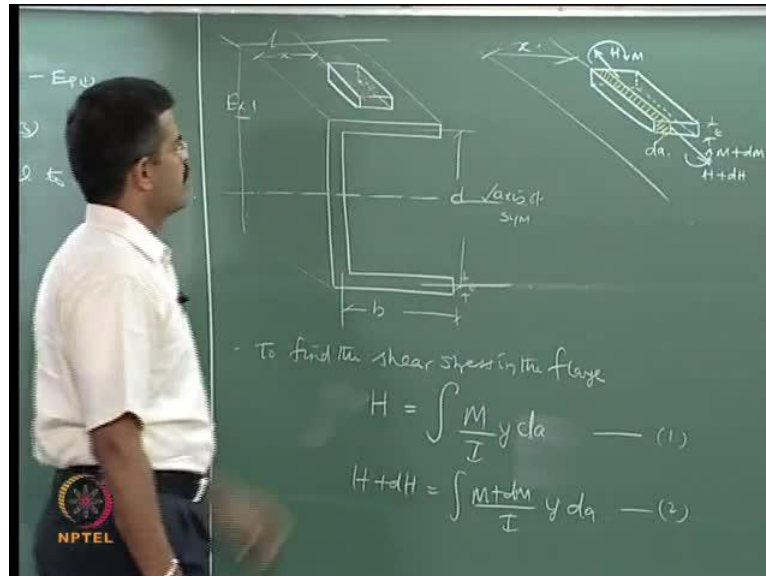
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Let us quickly recollect the shear centre is the point of intersection of the longitudinal axis of the member with the line of action of lateral loads, sorry transverse. So, if you have any cross section; this becomes a centroid of a cross section; this becomes my force acting; whereas this becomes the point of C which you called as shear centre and this is the point where W is acting I can say this as VR resultant shear force and this is W, and the distance between these two is what we call as e. So, it is understood that if W is not passing through the shear centre the section will be subjected to a twisting moment which I call as M_t it is given by W into e . Generally thin cylindrical sections where we are talking about asymmetric section commonly used in the marine structures. We know that they are very good in bending, but they are very poorly performing in torsion so this becomes the

problem that when the structure is subjected to twisting moment, one must examine this for the load satisfactory transfer in case of shear.

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So, we will take up another example today. We did one example of a channel section yesterday. We will take up another example of a channel section now and see how I can compute the shear centre, this is my channel section. Let us say the section has the breadth b and thickness t and the depth d of cross section as one axis of symmetry which is called as axis of symmetry. Now, consider an element here. This element is measured at a distance x here and if I draw element separately.

Now, understand this distance what we say as x , the element has a thickness t and the element has the forces acting like this. Let us say this is H and this is $H + dH$ and if this is M and this is $M + dM$ and of course I also consider an elemental area, this is da and so on. Now, let us say I want to find the horizontal shear force in the flange. We all know this the flange and this is the web. So, to find the shear force in the flange to start with let us say let us compute the horizontal force H here, H is M by I y da , dH is dM by I y da , whereas $H + dH$ is $M + dM$ by I .

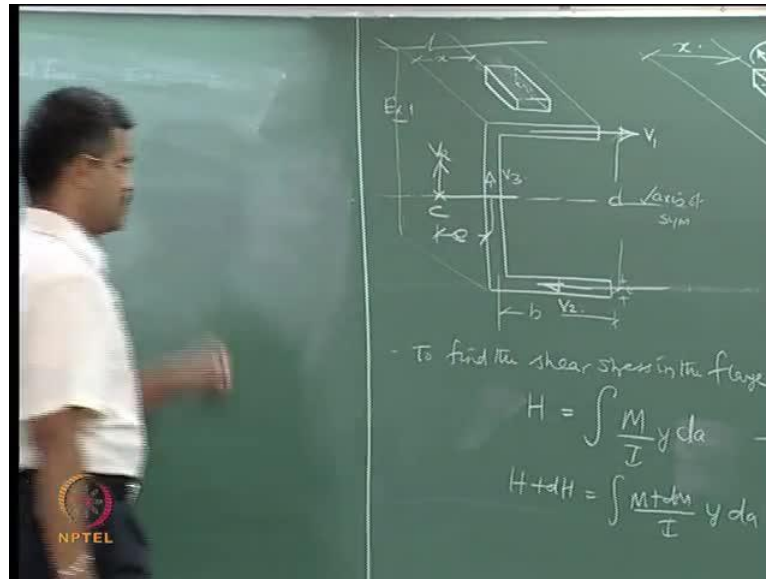
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dH , unbalanced longitudinal force = $E_1(u) - E_2(u)$
 $dH = \int \frac{dM}{I} y da$ — (3)
 for eqn, this unbalanced force should be equal to shear.
 $\tau(t dz) = \int_x^b \frac{dM}{I} y da$
 $= \frac{dM}{I} \int_x^b y da$
 $\tau = \frac{dM}{dz} \frac{1}{I t} \int_x^b y da$
 $\tau = \frac{V a \bar{y}}{I t}$

So, dH which is the unbalanced longitudinal force is given by let us say I call this equation number 1, equation number 2. I can say equation 2 minus equation 1. So, dH is simply integral of dM by I into y of da . Now, for equilibrium this unbalanced force should be equal to shear therefore, τ into the shear area which we can say in this case $t dz$ because I am measuring the thickness and I want the elemental strip should be considered in this section, let say a $t dz$ should be equal to let say integral dM by I into $y da$. Now, the limit what we have for this piece is varying from let us say x to b that is all it is.

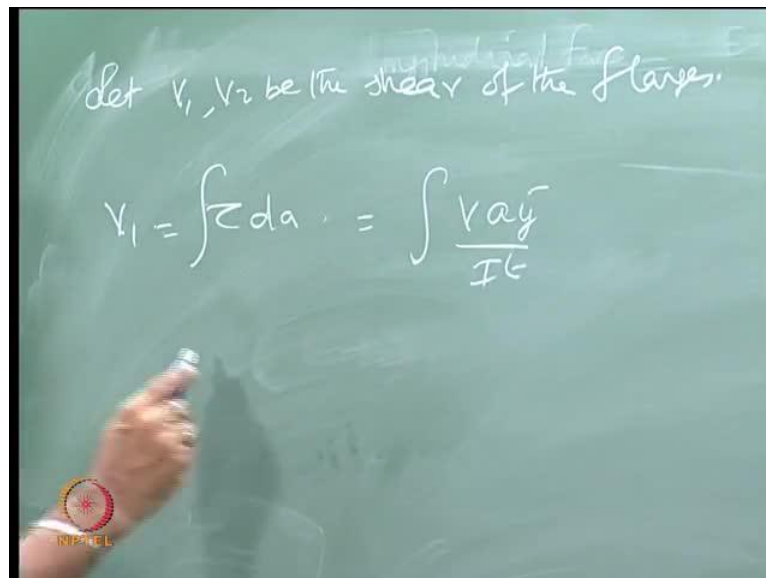
Okay, varies from x to b I am working about this so it varies from x to b . It starts from x and goes till b that is what the range of the pieces. So, this can now give me τ as I can rewrite this as dM by I $y da$. Therefore, τ can be we can say dM by dz 1 by I integral x to b $y da$. We all know that dM by dz will be V , integral da will be $a \bar{y}$ that is the first moment divided by $I t$, that (()) the equation which we used in the last section to compute the shear stress. So, in this case I is a moment of inertia of the whole section. Now, let us try to apply this principle back again from this problem and see how we can compute the shear centre. I will rub this, so this equation if you remember you have used, it is a general expression which we have used in the last lecture also.

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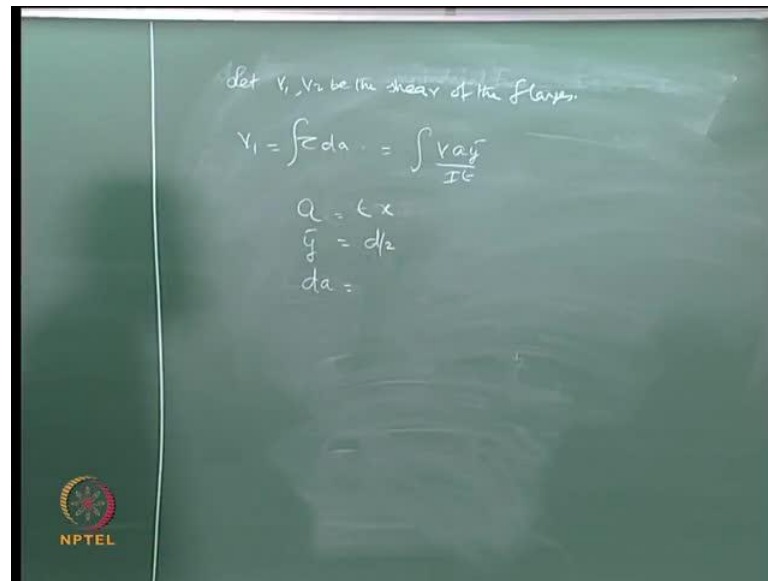
Let us say, in this problem I have the r is acting somewhere here and this is my shear centre C , this is V that is resultant and this measure from the face of the channel. And let us say this is my shear the upper one, this my shear draw here one and of course the shear here which you call as V_3 .

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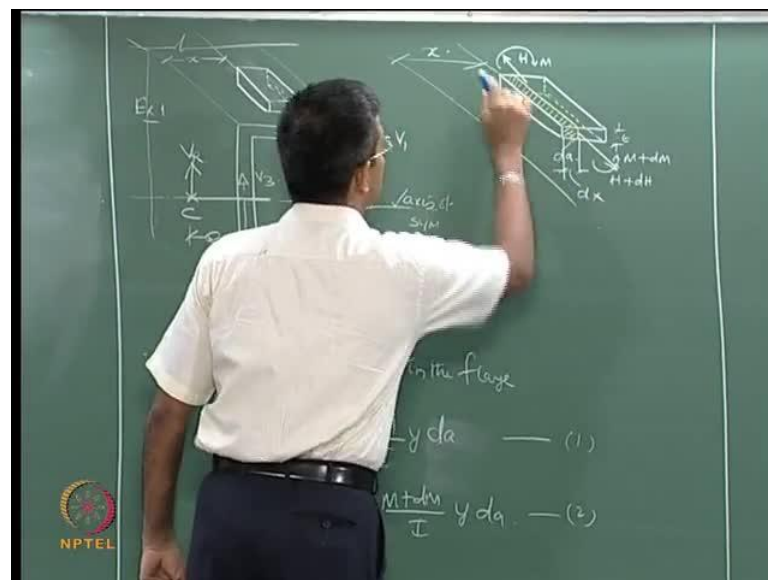
So, let V_1, V_2 be the shear of the flanges. Let V_1 now will be equal to simply τd as well which is $V a y \bar{I} t$. So, of course t is a thickness, we have written it here it is a thickness and the section is available already.

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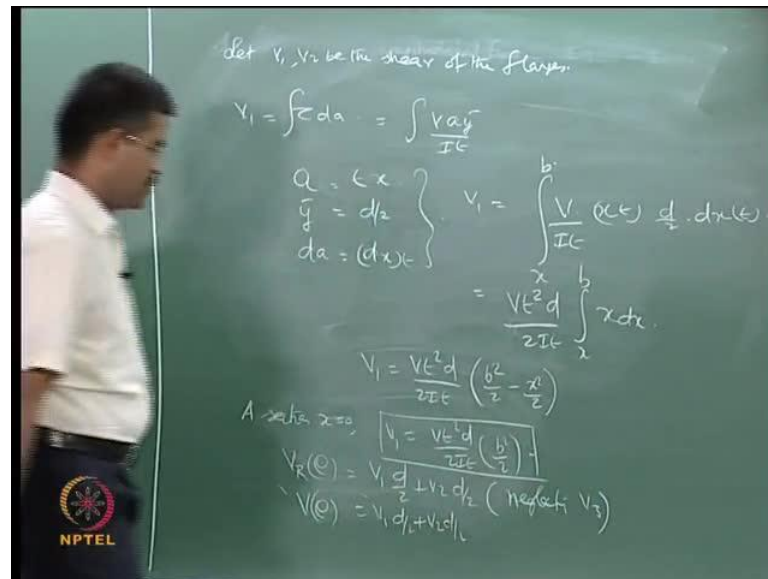
Let us see in my case is going to be t into x because I am considering a section which is x distance from here. So, t into xy bar is going to be distance of that from the centre, which will be $d/2$ and $d a$.

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So, the width of the strip say $d x$ and $d x$.

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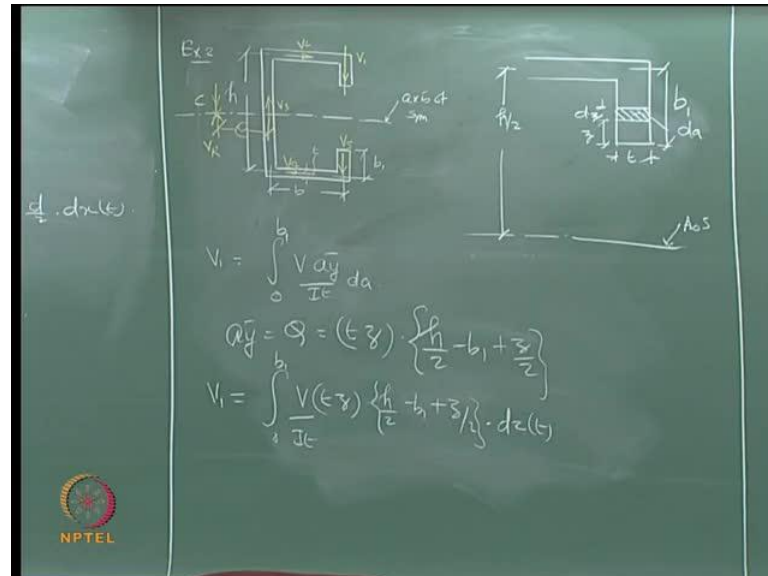
So, da is obviously $d \times t$ substituting back V_1 will be let us say $V_x t d$ by $2d \times b \times I$. And this variation is going to be from limits of x to b . So, integrate and tell me what the value of V_1 is? So, it is going to be $V t^2 d$ by $2I t$ of integral x to b , let us say $x dx$. So, I can simply say V_1 is going to be $V t^2 d$ by $2I t$ of b^2 minus x^2 . I want to consider the section where x is 0 here. So, section where V_1 will be $V t^2 d$ by $2I t$ of b^2 by 2.

Now, we all know that V_r is the resultant force which is going to be equal to the sum of V_1 plus V_2 . So, let us take the moment about this. So, I should say that V_r into e that is taking moment about this point will be equal to V_1 into d by 2 plus V_2 into d by 2 neglecting V_3 and we all know V_r is actually equal to the total shear in the cross section which is going to be $V_1 d$ by 2 plus $V_2 d$ by 2.

So, in this expression you know V , you know V_1 , because V_1 is given expression here because in V_1 we already know t d all are geometric dimensions we already know them, I is the moment of inertia the whole section, d is again a geometric parameter which is known from here, the only unknown in equation could be e that would locate the shear centre from the face of the section. So, this will give you the shear centre equation for easily for the channel section. Let us do one more example these are common sections

which are used for members of marine structures. So, we must know how to compute the shear centers of these.

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Let us take of another example where the slight deviation in the geometry with a box section. This is my section this has an axis of symmetry which marked here, an axis of symmetry, the dimensions are available. Let us say this is b and this is b_1 and of course the total thickness for the section is t and this dimension is h . So, I have the shear values, I call this shear as V_1, V_2, V_3, V_4 and V_5 and this is my resultant here; this my shear centre. The total shear forces acting here and this going to be V_R and I measuring e from the centre. That is my e . Let us take a specific case of this and try to work out V_1 .

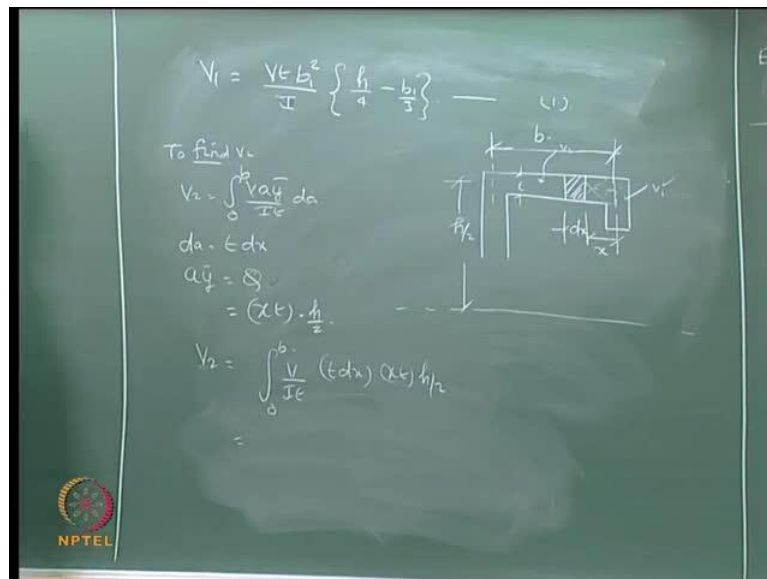
So, this is my piece, this is my axis of symmetry, I consider the piece on a specific thickness and we all know that this is going to be b_1 and of course, this value is b_1 , this value is t and this thickness is z and this thickness is $d z$. So, if you want to work out V_1 $V_1 = \int_0^{b_1} \frac{V \bar{y}}{I_x} t d a$ which varies from 0 to b_1 , t in this case is the thickness of the section considered, this my thickness remains and I is moment of inertia whole section and let us work out what is this \bar{y} which is otherwise called as capital Q in the literature.

Can you give me what is a \bar{y} ? Area a is an area above the section under consideration, \bar{y} is the c.g. of that distance from the section. So, a \bar{y} what is going to be a $\bar{y} = z$ yes, t and z and I want \bar{y} , what will be the \bar{y} value? Think about it and then tell

me. Yes, I want actually this distance is it not? This will be h by 2 minus b by 1 . Let us say, yes h by 2 minus b by 1 , good. So, we are here plus z by 2 is it not. This is, integrate this, yes, everything here so V_1 is going to be simply substitute these values and integrate them.

So, integration limits 0 to $b - 1$ plus z by 2 . Yes $d z$ is $d z$ that is $d a$, that is $d a$ of course, by I . $d a$ is $t d z$ is it not? This area, hatched area, this is what we call $d a$. Now, you have all the variables and you know the limits and you know the variables and integrate it, give me the value of V_1 . So, let me check this V by $I t z$ which by 2 minus V plus z by 2 $t d z$. Yes, what would be the value of V_1 ?

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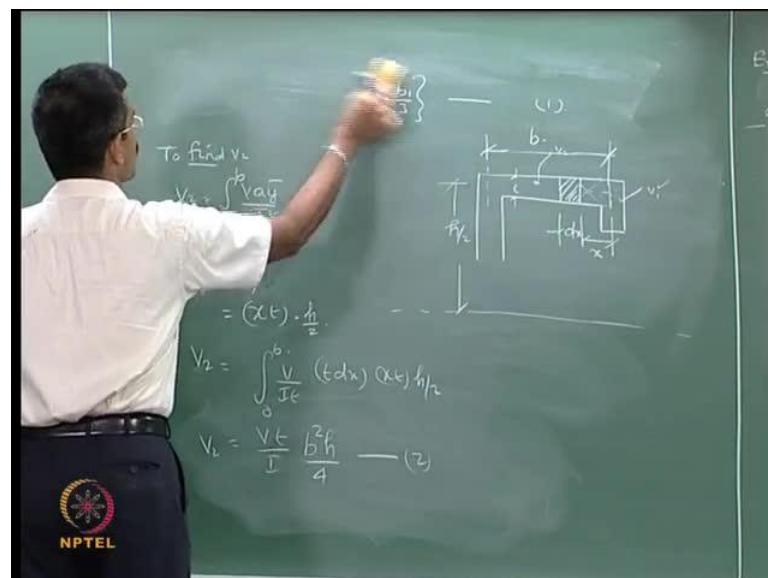
So, V_1 is given by have to substituting the limits for the equation it becomes V $t b^2$ square by I sorry by $I h$ by 4 minus b by 3 , call this equation number 1. Are you getting this? Let us do for the second piece which is V_2 . I will draw the figure again here to find V_2 . So, the figure is here, till the centre we already considered, this is V_1 I am looking from this centre till this centre for this piece that is V_2 .

Okay, so let me consider a section which is at a distance x from here and distance or thickness $d x$ and of course, this thickness we know it's t and this may act of symmetry and we know this distance is h by 2 and we already know from the figure this value is $b - 1$ square by I as b^2 minus b by 3 . So, what would be the $d a$ for this? V_2 is going to be integral of $V a y$ bar $I c$ $t d a$ let us say what is going to be $d a$ for this case, the

elemental area? Is going to be into $d x$ is it not? This $d x$ is t and what is a or what is y bar? That is nothing but Q , what is this value for this problem? I am looking for the strip here, this portion. So, x into t , is it not? And what is the y bar of that strip from the sub symmetry sorry h by 2 .

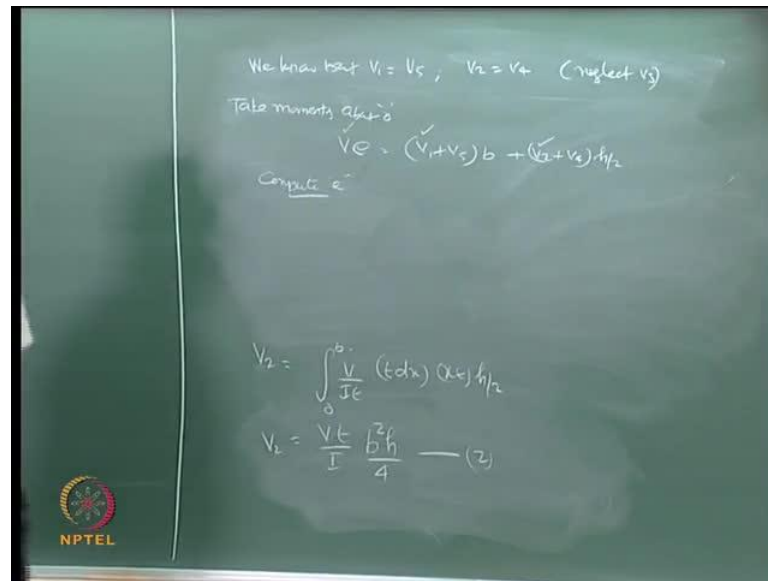
Why? This is already considered know, h by 2 till the centre is no minus 3 by 2 here till the centre, h by 2 . This portion is already considered, this is already done in V_1 is it not? So, we are only talking about V_2 . So, substitute back and what will be the limits for this? x is varying from, varying from 0 to b . Good, so substitute this and get me V_2 . So, V_2 is going to be integration of V by I t , t $d x$, x t h by 2 . Yes, 0 to b . So, this confirms to V t by I .

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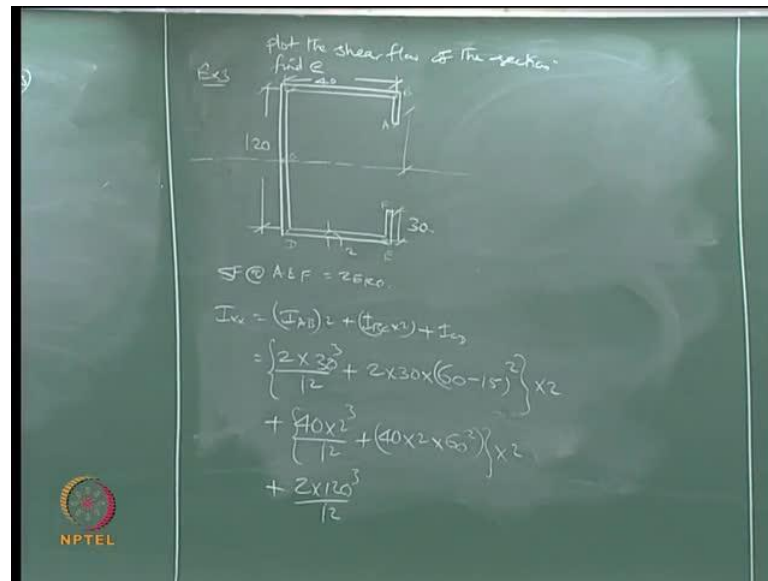
b square h by 4 , that is my V_2 , equation number 2. I think I will remove this; we will retain V_1 there so it is easy for us to write the expression.

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We already know that, we know that V_1 will be equal to V_5 and V_2 will be equal to V_4 . We will neglect web by the mean shear by the web. Let us take moment about this point. So, we into taking moments about this point, you can say V into e is V_1 plus V_5 of b , is it not? Plus V_2 plus V_4 of h by 2. So, in this case the total shear force resultant shear we know in this section V_1 and V_2 , we already have the expressions 1 and 2. V_4 and V_5 are same as them respectively, V and h are geometric dimension known to me, V_e can be computed. Compute, is it not? Which is a shear centre from the centre to the point C is what it is. We will quickly do one design problem and try to understand this, any doubt here?

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Let us say example 3 giving a problem I want you to do this quickly. My axis of symmetry what I want to plot is plot the shear flow of the section, that is one part and also find e , that is the distance of the shear centre for the given section. The section has dimensions like this; This is 40, all the dimensions and its centre, this is 40, this is 120 all the dimensions center to center is 120 and of course, this is 30 and the thickness is 2 through and through the constants. Let us designate these points, let us say this is my point A, point B, C, I call this as O, D, E and F we already know shear force flow at A and F are 0, why? Because a y bar will be 0 in this case, is it not?

There is no a y bar here therefore it is 0. Q value will be 0. So, I want to compute the shear force at b or the shear flow at b, but before that I want to do the moment of inertia of the entire section, is it not? So, let us try to find out moment of inertia of the entire section is let us say I a b into 2, is it not? I a b into 2, because I a b and EF are same plus IBC into 2 plus ICD, let us quickly find out this very simple, let us find out this. Yes, I a b, quick.

Okay 2×30^3 by 12 plus a k square that is $2 \times 30 \times 60^2$ now 30 is to the centre. Yes, in the original derivation b_1 is to the centre. I want the distance of this from here so what would be this? That is what I am interested in here k square, what would be that? $60 - 15$, is it not whole square. This is for AB of course, into 2. IBC 40×2^3 by 12 plus $40 \times 2 \times 60^2$ in the c g of this from here which is simply 60^2 , is it right? Into 2

plus 2 into 120cube by 12, this will give me I x axis, what is this value quick? Quick, quick.!

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Handwritten calculations on a chalkboard:

$$q_{12} = 0$$

$$q_{13} = q_{12} = \frac{VA\bar{y}}{I_x} = \frac{(25 \times 10^3) \cdot 30 \times 2 \cdot (60 - 30)}{11.07 \times 10^5 \times 2} = 29.67 \text{ N/mm}$$

$$q_{23} = q_{12} + q_{13} = 0 + 29.67 = 29.67 \text{ N/mm}$$

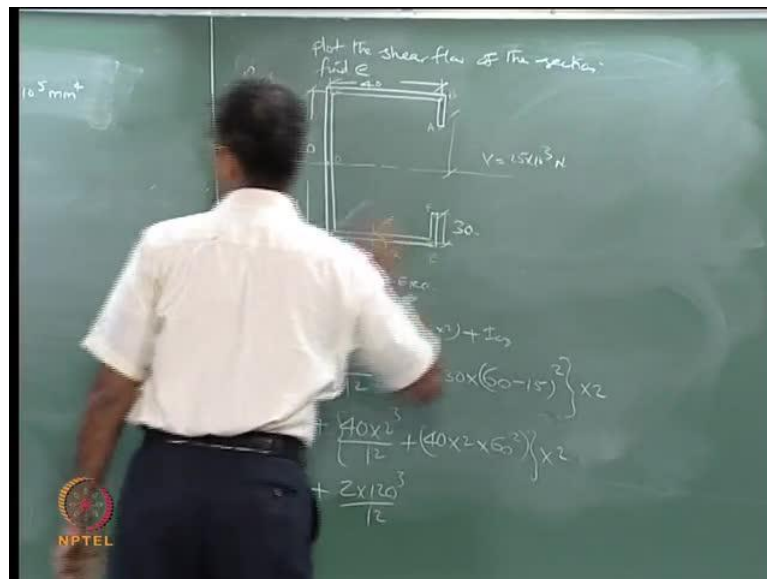
$$q_{34} = \frac{(25 \times 10^3) \cdot (40 \times 2) \cdot 60}{11.07 \times 10^5 \times 40} = 2.69 \text{ N/mm}$$

$$q_{45} = 29.67 + 2.69 = 32.36 \text{ N/mm}$$

$$q_{56} = 32.36 + \frac{25 \times 10^3}{(11.07 \times 10^5 \times 2)} \times (2 \times 60) \cdot 30 = 113.6 \text{ N/mm}$$

It is 2.4310 power 55.7610 power 5 is it 11.07 power 4, is it? Let us try to find a shear flow at point B q B at q A it is 0. So, let us find at q B shear towards B it will be VA y bar by I t.

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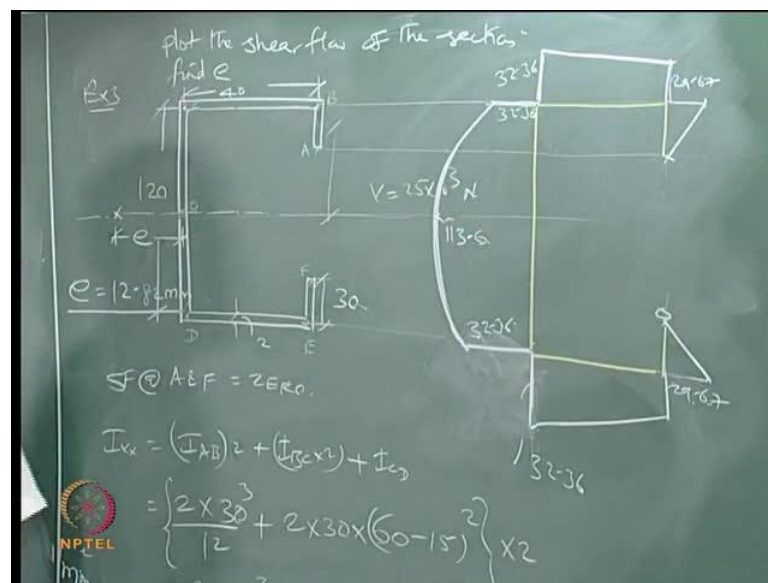


Okay, V in this problem is 25 kilo Newtons, is what? 25 into 10 power 3 a y bar of this which is 30 into 2 into 60 minus 30 by 2, that is may be a y bar, I is 11.07 into 10 power 5

thickness is 2. So, this value comes to 30 approximately the value is 29.67 Newton per m square which is as same as q . You want to find at q C which I should say q of BC plus q B, q at B. So, this piece plus shear at B that is what it is. So, which will be 2510 power 3 that is $V a y$ bar that is 40 into 2 that is a , and y bar is anyway going to be 60 divided by 11.07 into 10 power 5 into B^2 . How much is this? This is not 2, the width of this section is 40 is 40 . B here, the t here is the width at the section considered.

So, this value comes to 2.69 you can check that. So, this plus whatever I have that is 29.67 that is my q C. Let us talk about q o. Let us set this point here q o whatever value have here 29.67 plus 2.69 how much is this? This is 32.36. So, I should say 32.36 plus $V a y$ bar because of the section which is going to be 2510 power 3 by I is 11.07 into 10 power 5 into 2 that is $I t a y$ bar which will be 2 into 60 into 30 . So, this comes to 113.6 Newton per mm square.

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So, if I try to plot this. So, this point it is 0 and this point it is 29.67 and 29.67 then 32.36. Something, so, this is 32.36, this is 29.67, then similarly, here this point it is 0, 29.67, 29.67 and 32.36, variation at the centre is 113.6 from various. This is 113.6 this is 32.36 and 32.36. Say, this is my section which I am marking in yellow color. So, obviously this value becomes 29.67, this is 0, 29.67 and this value becomes 32.36. Of course, this is not correct. This value should be 32.36 and centre this value is 113.6. So, the variation is shown of the shear flow is shown like this.

Now,so we have completed discussions on shear centre.The next lecture we will talk about the combination of load effects and plastic effectson sections, plastic capacity of sections and then the collision loads and impact loads and marine structures.

Thank you.