

Introduction to Composites
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Lecture – 65

Physical Significance of Extensional Stiffness Matrix [A], Coupling Matrix [B] and Bending Stiffness Matrix [D] Matrices

Hello, welcome to Introduction to Composites. Today is the 5th day of the second last week yesterday we had defined 3 matrices, A matrix which is the extensional stiffness matrix, B which is the bend coupling matrix and D which is the a bending stiffness matrix. And these 3 matrices connect the force and moment resultants to midplane strains and midplane curvatures in a composite laminate.

Next what we will do today is we will discuss these matrices further and see what physical significance these matrices and specifically their individual elements carry.

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The slide contains the following handwritten content:

$$\begin{Bmatrix} N \\ M \end{Bmatrix} = \begin{bmatrix} A & B \\ B & D \end{bmatrix} \begin{Bmatrix} \epsilon^0 \\ \kappa^0 \end{Bmatrix}$$

— 6 equations

$$A_{ij} = \sum_{k=1}^n (\bar{Q}_{ij})_k (h_k - h_{k-1})$$

$$= \sum_{k=1}^n (\bar{Q}_{ij})_k t_k$$

$z = h_{k-1}$

k^{th} layer

$z = h_k$

t_k : thickness of k^{th} layer.

So, overall the relationship which we had developed was there is a n vector and there are moment resultant. So, they are related to this 6 by 6 matrix which is made up of A B and D sub matrices times midplane strains and midplane curvatures.

So, these are 6 equations, these are 6 equations. And the element; so we will consider each matrix term individually. So, first we will look at A ij which is the ij element of a

matrix this was defined as Q_{ij} for the k th layer times h_k which is the coordinate of the bottom portion of that layer minus h_{k-1} and we do this integral or summation from first layer to all the n layers, ok.

Now, this is let us say the k th layer, and this is my midplane. So, midplane z is equal to 0. So, here z equals h_k and this is z equals h_{k-1} . So, this difference is nothing but the thickness of k th layer and thickness of k th layer is always a positive number it is always a positive number. So, I can also write this as Q_{ij} for the k th layer times t_k and this goes from 1 to n Q_{ij} bar times t_k , where t_k is thickness of k th layer thickness of k th layer.

So, the point is that if Q_{ij} is positive and typically Q_{11} , Q_{22} , Q_{66} they are positive numbers always positive numbers, so in that case A_{11} , A_{21} , A_{22} , A_{66} they will always be positive ok.

Same thing was also true for Q_{12} , Q_{16} could be negative in that case A_{16} and A_{26} could be negative, but in all cases A_{11} , A_{12} , A_{22} and A_{66} will always be positive ok, they will always be positive because this t_k is always remains positive and Q_{11} Q_{22} Q_{16} Q_{12} and Q_{66} are always positive. So, respective elements in the A matrix are always positive, this is one thing.

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$$= \sum_{k=1}^n (\bar{Q}_{ij})_k t_k$$

$$B_{ij} = \sum_{k=1}^n (\bar{Q}_{ij})_k \left(\frac{h_k}{2} - \frac{h_{k-1}}{2} \right)$$

$$D_{ij} = \sum_{k=1}^n (\bar{Q}_{ij})_k \left(\frac{h_k^3}{3} - \frac{h_{k-1}^3}{3} \right)$$

t_k : thickness of k th layer.
 Any term B_{ij} can be -ive.
 may +ive
 may -ive
 Always positive

Next look at a B_{ij} ; So, in case of B_{ij} this is equal to $k \cdot 12 \cdot n \cdot Q_{ij}$ bar for the k th layer times h_k^2 by 2 minus h_{k-1}^2 by 2 square, ok. Now, in this case this term it may be positive or it may be negative also, it purely depends on the coordinates. Why because each of these terms is a positive number h_k^2 is positive h_{k-1}^2 square is a positive, but if h_{k-1} absolute value is much larger than that for h_k then this can be negative, it can be negative.

So, the point is that because of this thing this term in parentheses this can be either positive or negative. So, then what that means, is that any term of B_{ij} can be negative because consider at Q_{11} the B_{11} , B_{11} will depend on Q_{11} , Q_{11} is always positive, but if the terms in the parentheses are negative then it can be negative and finally, we look at D_{ij} . So, D_{ij} is k is equal to 1 to n , Q_{ij} for the k th of course, there is a bar times h_k^3 by 3 minus h_{k-1}^3 by 3.

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The image shows a whiteboard with the following handwritten content:

$$D_{ij} = \sum_{k=1}^n (\bar{Q}_{ij})_k \left(\frac{h_k^3}{3} - \frac{h_{k-1}^3}{3} \right)$$

Below the equation, there is a pink arrow pointing to the term in parentheses with the text "Always positive".

Below that, it says "and because $\bar{Q}_{11}, \bar{Q}_{12}, \bar{Q}_{66}, \bar{Q}_{22} > 0$ " and " $h_k > h_{k-1}$ ".

At the bottom, it lists " $D_{11}, D_{22}, D_{66}, D_{12} > 0$ ".

Now, this number is always positive. Why is it always positive? Because h_k , h_k is value because h_k is always more than h_{k-1} ok, it is always more than h_{k-1} . So, when I take the cube the difference will always be positive the same reason is for the same reason $h_k - h_{k-1}$ is always positive, ok.

So, this is always positive. So, for that reason and because Q_{11} bar, Q_{12} bar, Q_{66} bar and Q_{22} bar are always positive we can also say that D_{11} , D_{22} , D_{66} and D_{12} are

always positive. D_{16} and D_{26} can be negative because depending on the value of θ I can have negative values for those terms also.

So, the point tells that these terms are always positive, similar terms in the A matrix are also positive, but in terms of in context of B matrix any term can be positive or negative and it is given driven by the fact that h_k^2 minus h_{k-1}^2 can be either positive or negative. So, this is one general observation I wanted to state.

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$$\begin{Bmatrix} N \\ M \end{Bmatrix} = \begin{bmatrix} A & B \\ B & D \end{bmatrix} \begin{Bmatrix} \epsilon \\ \kappa \end{Bmatrix}$$

CASE 1 $[B] = [0]$

$$\begin{aligned} \rightarrow \{N\} &= [A]\{\epsilon\} \\ \{M\} &= [D]\{\kappa\} \end{aligned}$$

only strains

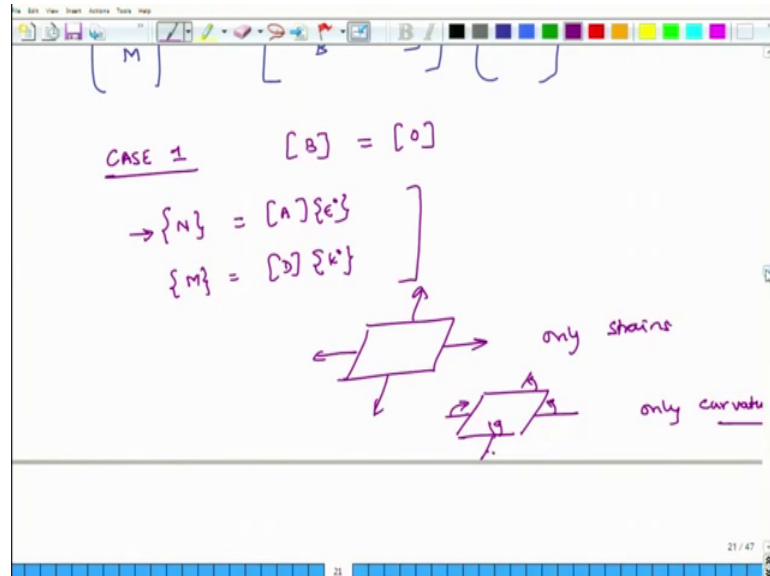
Next look at some more details about these matrices. So, if we have n and n these are the vectors, so this is equal to A B B and D these are sub matrices ϵ κ . So, these are midplane stresses, a midplane strains and midplane curvatures. So, we will consider several cases case 1.

We can have a lamination sequence we can have a laminate such that and we will discuss the conditions when that happens maybe later. So, there could be a situation that then B matrix all the elements in B matrix are 0, all the elements in the B matrix are 0. Then N equals A times ϵ and M equals D times curvatures ok.

So, what this tells us is that if for a laminate the B matrix is 0 then if I apply only forces on the edge of the plate if I apply only forces on the edge of the plate. So, I am only stretching it suppose I am not applying any moments I am not trying to bend and twist it,

then it will generate only strains it will not generate curvatures and if I apply only moments if I apply only moments then I generate only curvatures, ok.

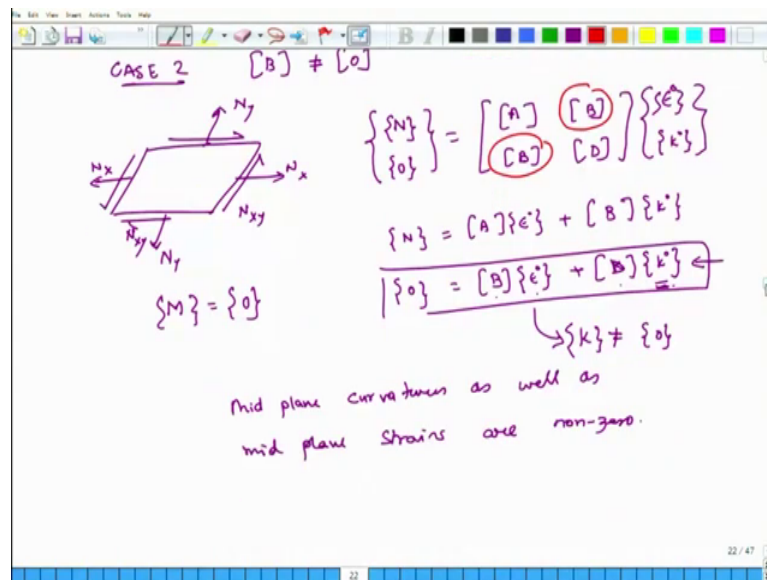
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So, what this says is that if the B matrix for a laminate is 0, force resultants generate only midplane strains and they do not generate midplane curvatures and moment results only generate midplane curvatures and they do not generate any midplane strains this is what it means ok. It is very important to understand that.

In lot of isotropic materials the B matrix is not lot all isotropic materials made up of isotropic systems this is actually the case where you have in the lot of systems B matrix is 0 and so extensional forces only generate strains and moments only generate curvatures they do not generate midplane strains and vice versa. So, this is something important to understand.

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But we can have another case where B matrix is not of 0 matrix, B matrix is not a 0 matrix and suppose in this case you have let us say a plate and you apply some N_x . you can also apply some N_y and you can also apply some N_{xy} .

And let us say that you do not apply any moments. So, you say that moment we do not apply any moments, but then what do the equations tell us the N matrix is nonzero right and in this plate I am applying 0 moments external moments. So, this is 0. So, this is equal to A B B and D midplane strains curvatures. So, we want to figure out what kind of strains and curvatures are in this plate.

Now, in this case because the B matrix is not 0 we cannot say for sure that midplane curvatures will be nonzero ok, because when I write 2 individual sets of equations what do I get N equals A epsilon plus B times curvature and for because moments are 0 external moment resultants are 0. So, this is 0 is equal to B times midplane strains plus D times midplane curvatures.

The second equation tells us that for this system curvatures are nonzero because the sum of B times epsilon plus D times curvatures is 0. So, second equation tells us that curvatures are not 0 and we have to solve all these 6 equations to find curvatures as well as midplane strains. So, in this case midplane curvatures as well as midplane strains are nonzero and this is all happening because of the B matrix.

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CASE 2 $[B] \neq [0]$

Diagram of a rectangular plate with forces N_x , N_y , and N_{xy} .

$\{M\} = \{0\}$

$$\begin{Bmatrix} N_x \\ N_y \\ N_{xy} \end{Bmatrix} = \begin{bmatrix} [A] & [B] \\ [B] & [D] \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \kappa_x \\ \kappa_y \end{Bmatrix}$$

$$\{N\} = [A]\{\epsilon\} + [B]\{\kappa\}$$

$$\{M\} = [B]\{\epsilon\} + [D]\{\kappa\}$$

Mid plane curvatures as well as mid plane strains are non-zero.

$$\begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \kappa_x \\ \kappa_y \end{Bmatrix} = \begin{bmatrix} \\ \\ \\ \end{bmatrix} \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \kappa_x \\ \kappa_y \end{Bmatrix}$$

Similarly, we can have another situation where we have no force resultants, but we have only moment resultants and in this case also we will find that both midplane curvatures and midplane strains will exist.

So, what this tells us is that it is the B matrix if it is nonzero then it generates even though if we are just stretching if we are just pulling the plate in a straight direction even then it will not only expand and contract, but it will also bend and that is why it is called coupling matrix.

That is why it is called coupling matrix, because it couples the midplane response of the plate to the midplane curvature of the plate it couples that ok. If this coupling matrix is not there then the response of the plate to M is purely strain response of plate to M is purely curvature, but if B is nonzero then it couples and you have a more complicated situation. So, this is the second case, the second.

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The slide content is as follows:

At the top, a handwritten equation: $\begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \gamma_{xy} \end{Bmatrix} = [A] \begin{Bmatrix} N_x \\ N_y \\ N_{xy} \end{Bmatrix}$

CASE 3 Assume $[B] = [0]$

The matrix equation is written as: $\begin{Bmatrix} 0 \\ 0 \\ N_{xy} \end{Bmatrix} = \begin{bmatrix} A_{11} & A_{12} & A_{16} \\ A_{12} & A_{22} & A_{26} \\ A_{16} & A_{26} & A_{66} \end{bmatrix} \begin{Bmatrix} \epsilon_x^0 \\ \epsilon_y^0 \\ \gamma_{xy}^0 \end{Bmatrix}$

Below the matrix, the terms A_{16} and A_{26} are circled in red. The text below reads: A_{16} and A_{26} couple extension and shear strains. A_{16} & A_{26} = Extensional-shear coupling coefficients.

A diagram of a rectangular plate is shown with forces N_x , N_y , and N_{xy} applied to its edges. Strains ϵ_x , ϵ_y , and γ_{xy} are indicated on the plate.

Let us look at some more cases. So, there is a third case ok, third case. And here we look at the a matrix in detail. So, we assume that we have designed the plate such that it is B 0, you have designed such that it is B 0 and we are loading the plate only in using N. So, the plate is experienced in N x and y and maybe also N xy, ok.

So, if B is 0 then the plate will not have any midplane curvatures because the bending the coupling matrix is 0, but then let us look at the equations. So, the equation will be N x, N y, N xy and that equals epsilon x naught, epsilon y naught, gamma xy naught. And here it will be A 11, A 12, A 16, A 22, A 26 and A 66, A 12, A 16, A 26.

Now, consider the case that we are applying only N x in the situa; in the on the plate suppose I apply only N x on the plate. So, if I apply only N x on the plate then N y and N xy is 0, ok. If A 16 and A 26 terms are not 0 then even though I am pulling the plate in the straight direction only in extensional mode the plate will also develop a shear strain because A 16 and A 26 are not 0, ok.

So, A 16 and A 26 couple extensional extension and shear strains, if A 16 was 0 and A 26 was 0 then if I pull it, it will only exhibit stretching and Poisson contraction, but if A 16 A 26 are not 0 and if I pull it will also generate shear strain.

Similarly, if I applied only shear stress you know N xy suppose I applied only N xy, if I applied only N xy and if A 16 and A 26 are not 0 then these terms because they appear in

the matrix here they again couple epsilon x, epsilon y and gamma xy. So, even though I am applying pure shear forces on the plate it is not only experiencing shear strains, but it is also getting stressed and compressed ok.

So, that is why A 16 and A 26 are extensional shear coupling coefficients, the couple the shear response and the stray extensional response ok. And if they are 0 then the plate if it experiences purely tensional strain then it has only extensional strains if it experiences purely shear stresses then it exhibits only shear stresses.

Now, of course, this is in the context that B matrix is 0, ok. And we look at a 4th case.

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The slide shows the following handwritten content:

CASE 4 $[B] = 0$ Bending response of plate.

$$\begin{Bmatrix} M_x \\ M_y \\ M_{xy} \end{Bmatrix} = \begin{bmatrix} D_{11} & D_{12} & D_{16} \\ D_{12} & D_{22} & D_{26} \\ D_{16} & D_{26} & D_{66} \end{bmatrix} \begin{Bmatrix} K_x^0 \\ K_y^0 \\ K_{xy}^0 \end{Bmatrix}$$

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We look at a 4th case. So, again for simplification purposes we still assume that B is equal to 0 and I want to see the bending response of the plate. So, I am interested in seeing the bending response of the plate. So, what do I see? So, in that when B matrix is 0 then M_x, M_y, M_{xy} equals $D_{11} D_{12} D_{16}; D_{12} D_{22} D_{26}; D_{16} D_{26} D_{66}$ this is K_x, K_y and K_{xy} , ok. So, if B matrix is 0 then if I am applying only moments on the system then these are the equations. If N is 0 then the first 3 equations are irrelevant and if because only moments are being applied moment resultants are being applied.

Again we should understand what are K_x, K_y and K_{xy} . So, what I will do is I will again show you this book, and suppose this is the x axis suppose this is the x axis this is the y axis and downwards is the z axis if I bend the book like this then the plate or the book is

exhibiting purely K_x , it is exhibiting purely K_x curvature in the x direction it does not have curvature in the y direction when I bend it like this because when you look at it it is still straight. So, it is not having any curvature in the y direction

If I bend it like this then it is having purely curvature in the y direction. So, if I bend it like this it is having pure K_y , if I bend it like this it has pure K_x , K_y is 0 if I bend it like this it is having pure K_y , K_x is 0. And then there is something called K_{xy} and what is that? So, this is bending this is also bending, but then I can also twist it like this.

In this case it is having curvature in this direction also, it is having curvature in that direction also it is having curving and it is also a cross components ok. So, you can have a plate which has curvature in this direction, that direction and also a cross component. So, so w the slope is changing, the $w_{,xx}$, $w_{,yy}$ second derivative of w with respect to x and y is nonzero ok. So, that is called twisting that is called twisting. So, if I am trying to twist it or something like this. So, that is the twisting thing.

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CASE 4 $[B] = 0$ Bending response of plate.

$$\begin{Bmatrix} 0 \\ 0 \\ M_{xy} \end{Bmatrix} = \begin{bmatrix} D_{11} & D_{12} & D_{16} \\ D_{12} & D_{22} & D_{26} \\ D_{16} & D_{26} & D_{66} \end{bmatrix} \begin{Bmatrix} K_x \\ K_y \\ K_{xy} \end{Bmatrix}$$

K_x → Bending
 K_y → Bending
 K_{xy} → Twisting

$D_{16} \neq 0$: Pure Bending moments → generate twisting curvature as well.
 $D_{26} \neq 0$: Pure twisting moment → generate bending moments as well.
 D_{16}, D_{26} ∴ Bending-twisting coupling coeff.

So, this is a bending curvature this is a bending curvature and this is a twisting curvature, is the twisting curvature. Now, consider a situation where I apply only M_x , and I do not apply M_y and M_{xy} , so M_y and M_{xy} are 0. In that case if these terms are nonzero D_{16} and D_{26} then pure bending moments. So, M_x is a purely bending moment they generate twisting curvatures as well ok, because D_{16} and D_{26} is not 0. Similarly, if I apply a

pure twisting moment; So, what is the twisting moment? M_{xy} , M_{xy} is a pure twisting moment because I am trying to twist it.

So, pure twisting moments these guys they generate bending moments as well ok, because of if D_{16} and D_{26} are not 0. So, this happens when D_{16} is not 0 or D_{26} is not 0 and typically we do not want our structure to twist because it then tries to shear off easily. So, that is why these terms D_{16} and D_{26} couple the bending and twisting response of the plate and that is why D_{16} , D_{26} they are called bending twisting coupling coefficients, ok.

So, we have discussed 4 cases the first case is when B is 0, and if $B = 0$ then the coupling between the bending response and the midplane strain response extensional response and the bending response it gets eliminated that is why it is known as coupling matrix.

The second, so that was about case 1 and case 2 then we looked at the case when A_{16} and A_{26} are not 0, and in this case the extension and the shear strains they get coupled that is why A_{16} and A_{26} are known as extensional shear coupling coefficients. In the last case is when D_{16} and D_{26} are not 0 then bending and twisting response of the plate they get coupled. So, that is why it is known as bending twisting coupling coefficient.

So, this concludes our discussion. Tomorrow we will have another discussion on lamination sequence and how lamination sequence is written using standard lamination code. So, that is what we plan to do tomorrow until then have a great time.

Thank you.