

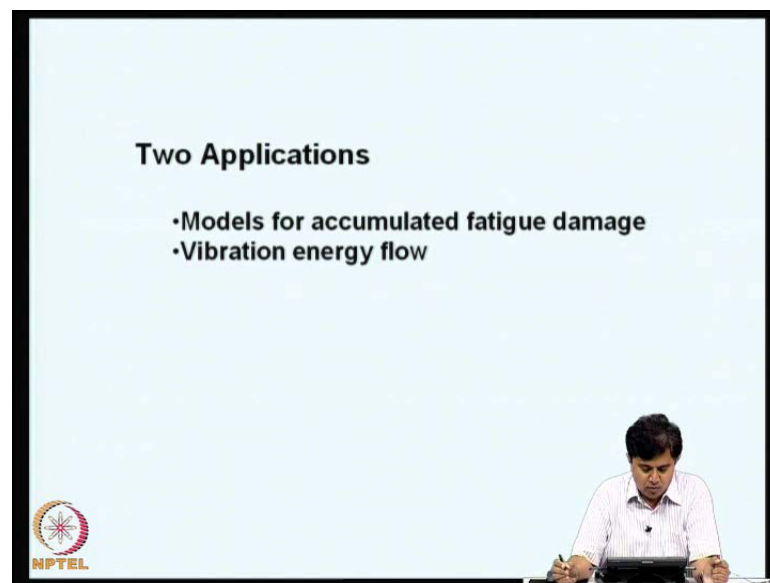
**Stochastic Structural Dynamics**  
**Prof. Dr. C. S. Manohar**  
**Department of Civil Engineering**  
**Indian Institute of Science, Bangalore**

**Module No. # 09**

**Lecture No. # 36**

**Fatigue Failure & Vibration Energy Flow Models**

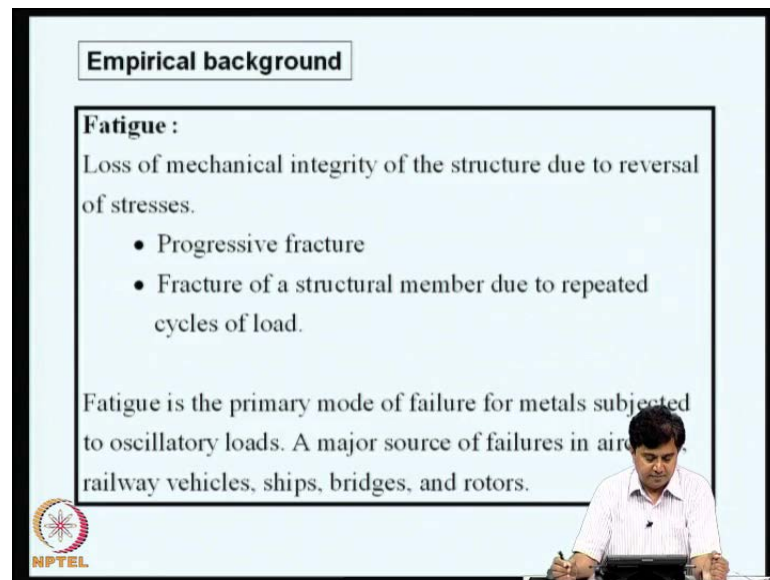
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We have been discussing an application of principles of random vibration to problems in earth quake engineering. We will continue with discussion on applications. So, in this lecture, I will consider two more applications - one is in the area of models for accumulated fatigue damage in randomly vibrating structures, and the second one is the area of vibration energy flow that is important in the area of high frequency vibrations of engineering structures.

So, what I intend to do is to be favor of these two applications areas and illustrate how principles of random vibration analysis are employed here. The objective is not to describe the problems of fatigue damage nor the problems of energy flow, but to simply state the basic problem and illustrate how random vibration principles could be used.

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**Empirical background**

**Fatigue :**  
Loss of mechanical integrity of the structure due to reversal of stresses.

- Progressive fracture
- Fracture of a structural member due to repeated cycles of load.

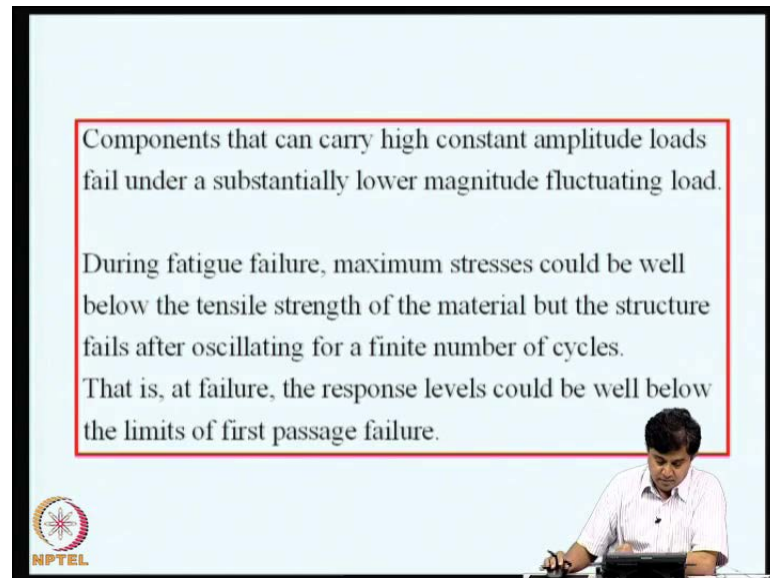
Fatigue is the primary mode of failure for metals subjected to oscillatory loads. A major source of failures in aircrafts, railway vehicles, ships, bridges, and rotors.

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So, we will start with problem of fatigue damage accumulation. So, some, we start with some empirical background. What is fatigue? So, we can say the fatigue is loss of mechanical integrity of the structure due to reversal of stresses. So, any structural material on some scale, on any scale, on, even on very fine scales will have some imperfections defects. Consequently, the strength of the structural material in a given volume depends on amount of defect that are present.

Now, when the material is subjected to vibratory loads, there will be reversal of stresses, and as a consequence of this, the defects and imperfections grow within a given volume of structural material, and consequently, the strength of the material reduces and this phenomena is what is known as fatigue. So, fatigue is a essentially a of phenomena involving of progressive fracture. So, it is a fracture of a structural member due to repeated cycles of load. Fatigue is the primary mode of failure for metals subjected to oscillatory loads. So, this is a major source of failures in aircrafts, railway vehicles, ships, bridges and rotors.



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Components that can carry high constant amplitude loads fail under a substantially lower magnitude fluctuating load.

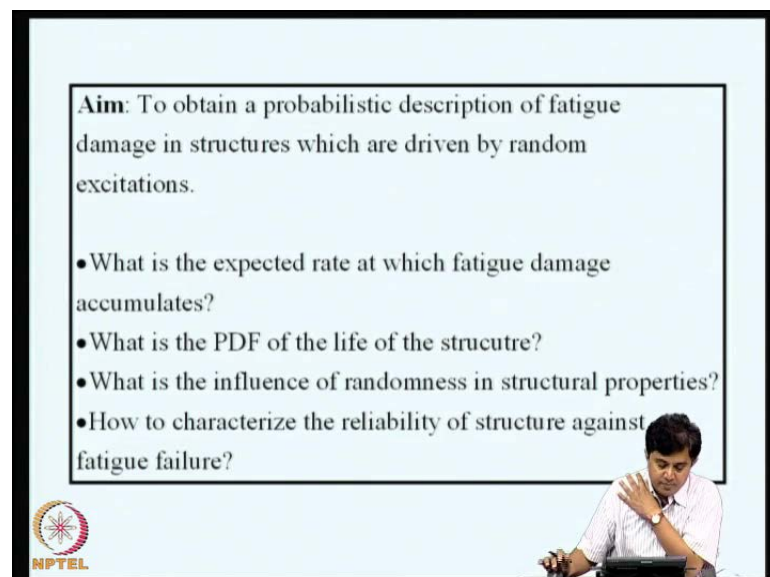
During fatigue failure, maximum stresses could be well below the tensile strength of the material but the structure fails after oscillating for a finite number of cycles.

That is, at failure, the response levels could be well below the limits of first passage failure.



Actually structural components that can carry high constant amplitude loads fail under a substantially lower magnitude fluctuating load. This is due to, basically due to fatigue, and during fatigue failure, maximum stress could be well below the tensile strength of the material but the structure fails after oscillating for a finite number of cycles, that is, at failure, the response level could be well below the limits of first passage failure.

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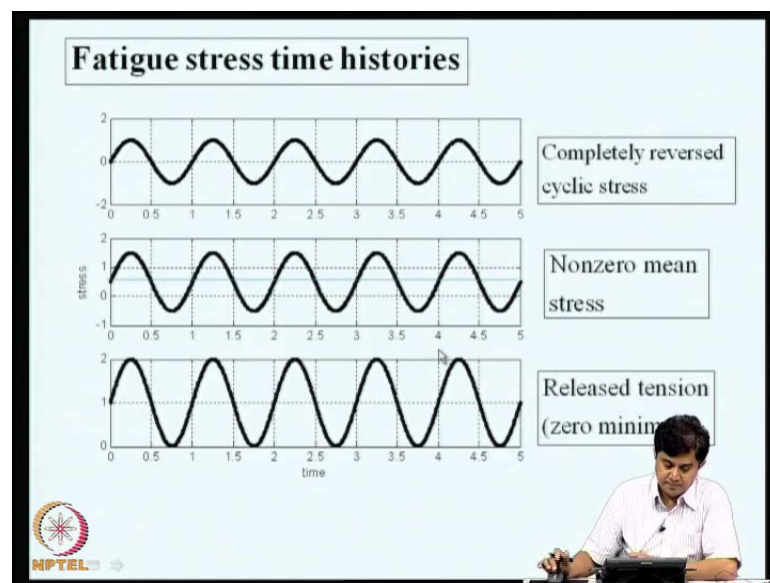
**Aim:** To obtain a probabilistic description of fatigue damage in structures which are driven by random excitations.

- What is the expected rate at which fatigue damage accumulates?
- What is the PDF of the life of the structure?
- What is the influence of randomness in structural properties?
- How to characterize the reliability of structure against fatigue failure?

So, it is a failure could be quiet catastrophic. So, it is important to obtain a rational description of how to model these failures. So, the objective of this discussion is to obtain a probabilistic description of fatigue damage in structures which are driven by random excitations. For example, the types of questions that would like to ask are - what is the expected rate at which fatigue damage accumulates? What is the probability distribution of the life of the structure? What is the influence of randomness in structural properties? How to characterize the reliability of structure against fatigue failure? So, we will consider some of these questions and see how we can progress.

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Now, if you look at time histories of a stresses, the various possibilities exist. For example, in this first one, there is an oscillatory load about a mean level. That would mean the stress cycles are completely reversed and these are called completely reversed cyclic stresses. This oscillation could occur about a non-zero mean. So, the cycling occurs about a non-zero mean which is not 0 or it could be such that there is no stress reversal, but actually there is a for instance released tension. Here, the stress never enters the compression zone. It remains tensile but the tensile stress magnitude keeps oscillating.

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$S_{\max}, S_{\min}, S_{\text{mean}}$  = maximum, minimum and arithmetic mean of stress

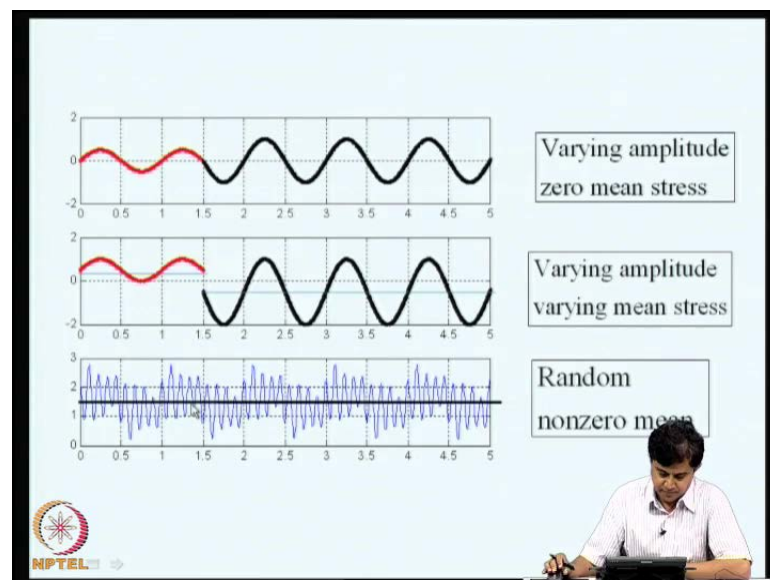
$\Delta S = S_{\max} - S_{\min}$  = stress range

$S_a = \frac{\Delta S}{2}$  = alternating stress amplitude

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Now, here, we introduce certain terms  $S_{\max}$ ,  $S_{\min}$  and  $S_{\text{mean}}$  are the maximum, minimum and arithmetic mean of these stresses.  $\Delta S$  is a stress range  $S_{\max} - S_{\min}$  and the alternating stress amplitude is given by  $\Delta S$  divided by 2.

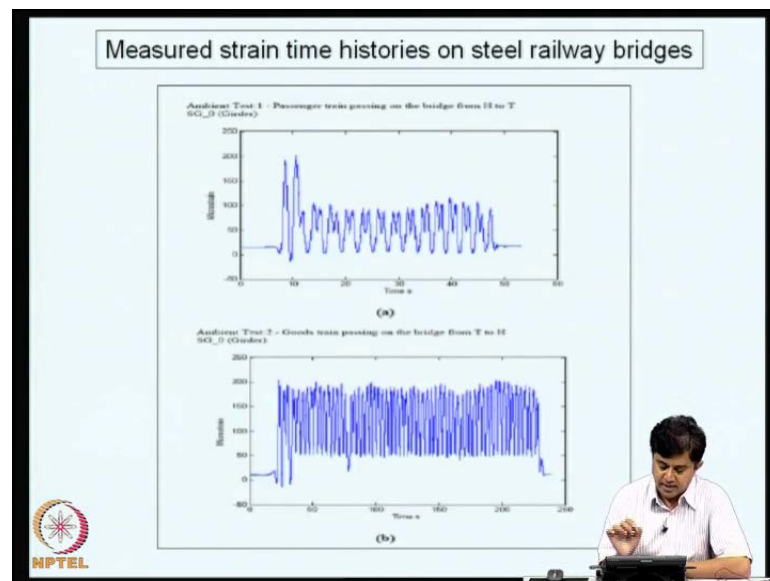
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The stress amplitude need not be harmonic. The amplitude could vary. For example, here, there is a stress time history where up to say 1.5 second there is one amplitude. Beyond that, the amplitude increases.

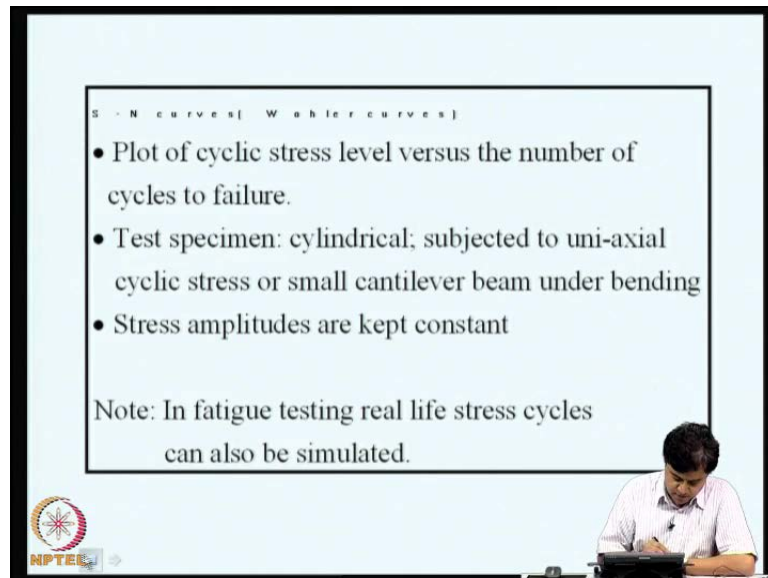
Now, the change in amplitude could also be accompanied by change in the mean levels about which these oscillations occur. These are varying amplitude, varying mean stresses, or all the stress could be a randomly varying in time. It could be multi frequency. The, here, what we are shown is a narrow band random process. So, here, there is no, the definition of a cycle here is not immediately evident unless we clarify what exactly that is.

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Just we will illustrate here I am showing certain measure time histories. For example, in this graph, we are showing the time history of the strain on a steel girder bridge as a train formation crosses the bridge at a particular velocity. So, this is second graph is similar result when a longer formation may be free traffic crosses the bridge. So, the cycle that we see here typically corresponds to the passage of one locomotive and carriage wagon and this wagon themselves we will have multiple axles and these small oscillations about a higher level represent, you know, oscillation due to passage of axils of the same wagon.

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S - N curves | W a h l e r c u r v e s |

- Plot of cyclic stress level versus the number of cycles to failure.
- Test specimen: cylindrical; subjected to uni-axial cyclic stress or small cantilever beam under bending
- Stress amplitudes are kept constant

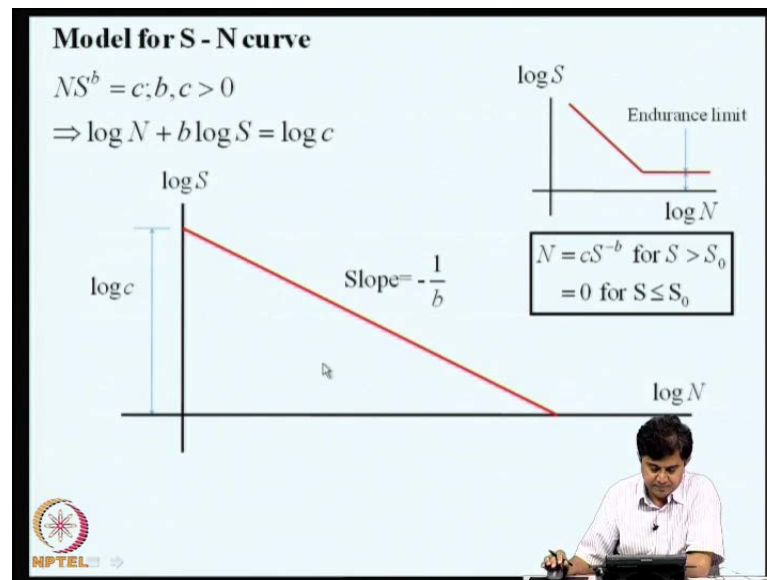
Note: In fatigue testing real life stress cycles can also be simulated.

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Now, we use what is known as S N curves to characterize fatigue failure. This S N curve essentially is a plot of cyclic stress level versus number of cycles to failure; that means if you take a test coupon and subject it to cyclic loaded into a given stress value after certain number of cycles of oscillation, this coupon will fail and the number of oscillation as a function of stress level is known as a S N curve. So, this is something that is experimentally obtained. This also is known as Olar curve.

The test specimen is typically a cylindrical specimen subject to uni-axial cyclic stress or a small cantilever beam under bending oscillation. The stress amplitudes are kept constant. We could also similar actual real life stress cycles in a fatigue test but we can, this is not what we are discussing in this lecture.

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A simple model for this so called S N curves is of this form  $N S^b = c$  - where  $b$  and  $c$  are constants, and if you take every term of this, we get  $\log N$  plus  $b \log S$  is equal to  $\log c$ . That would mean on log scale, S N curve is a straight line with slope minus 1 by  $b$  and intersect  $\log c$ .

Now, in reality, what happens is - below a particular stress level, there would not be failure due to cyclic oscillation is not possible and this limit of stress is known as endurance limit, and we are considering stress levels above the endurance limit in our studies. So, number of cycles from this formula  $N S^b = c$ . We can, in, for this case, it is  $N = c S^{-b}$  for  $S > S_0$  is 0 for  $S \leq S_0$  if you taking to account the effect of endurance limit.



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Remarks

- Does not deal with physical phenomena within material.
- It does not separate the crack initiation and crack propagation stages.
- Considers only the total life to fracture.

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Now, in the S N curve approach, it is an integral approach which actually does not deal with physical phenomena that would be taking place within the material which results in loss of mechanical integrity. Actually does not separate the crack initiation and crack propagation stages, considers only the total life to fracture.

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Laboratory conditions → Field conditions

**Factors**

- Nonzero mean stress
- Varying stress amplitudes
- Environmental conditions: temperature, humidity, corrosive media...
- Size, shape and surface finish
- Frequency of cycling

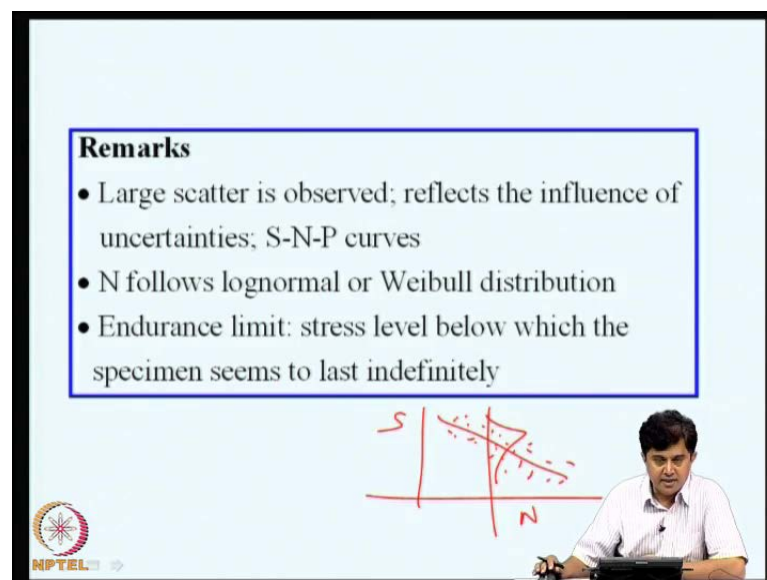
The slide features a light blue background with a black border. At the top, a box labeled 'Laboratory conditions' has a red arrow pointing to a box labeled 'Field conditions'. Below this, a larger box contains the text. In the bottom right corner, there is a small inset image of a man in a white shirt sitting at a desk with a laptop. The NPTEL logo is visible in the bottom left corner.

Now, we obtain S N curves in the laboratory conditions and then we want to extrapolate this information to the field conditions. So, laboratory testing is done on coupons just like with the way we find Young's Modulus of a, say material using coupons. In the

same sense, S N curves are also obtained in a laboratory conditions on test coupons, but these results need to be extrapolated to free condition.

When we do that, there are factors like nonzero mean stress, then varying stress amplitudes, an environmental conditions like temperature, humidity, corrosive media, etcetera will start influencing the results, and also the size shape and surface finish of the member under consideration also have significant influence on the fatigue performance. The frequency of cycling could also important but many times this is not a very crucial factor.

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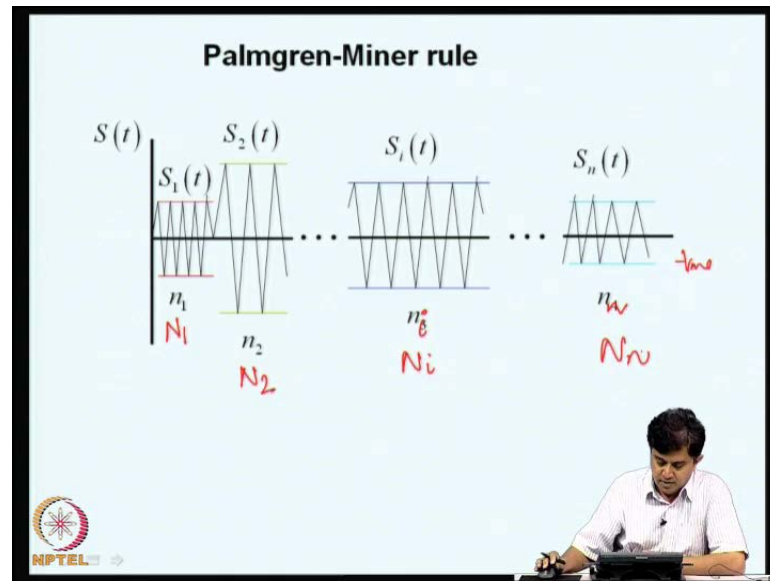
**Remarks**

- Large scatter is observed; reflects the influence of uncertainties; S-N-P curves
- N follows lognormal or Weibull distribution
- Endurance limit: stress level below which the specimen seems to last indefinitely

Below the text box, there is a hand-drawn graph in red ink. The vertical axis is labeled 'S' and the horizontal axis is labeled 'N'. The graph shows a downward-sloping line with several data points scattered around it, representing the scatter in S-N curves. In the bottom right corner of the video frame, a man in a white shirt is visible, sitting at a desk and looking towards the camera. In the bottom left corner, there is a logo for NPTEL.

In the experimental data, actually large scatter is observed is reflects the influence of uncertainties, and one actually plots, for example, the S N curve, one gets an, if, in experiment there where green straight line would not emerge. So, at any given point, you can draw a probability distribution curve, density function, and one can specify the S N curves in terms of S N and probability curves. This density function could be modeled as lognormal or Weibull. Endurance limit have already explained; it is a stress level below which the specimen seems to last in definitely.

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Now, we use what is known as Palmgren Miner rule to find out accumulation of a fatigue damage. Suppose during the life of the structure on x axis, I am plotting time, and on y axis we are plotting stress amplitudes. Suppose the structure undergoes  $N_1$  oscillations at stress level  $S_1$  ( Refer Slide Time: 11:00) and  $N_2$  oscillations that stress level  $S_2$  and  $N_3$  at  $N_i$ ;  $N_i$  at stress level  $S_i$  and  $N_N$  at stress level  $S_N$ . Now, if the specimen is left to oscillate stress level  $S_1$ , it will required as a capital  $N_1$  number of cycles for failure. Similarly, if the specimen is left to oscillate only at  $S_2$ , it may required  $N_2$  number of cycles to failure and these capital  $N_1$  capital  $N_2$  etcetera are obtain from the  $S_N$  curve.

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Stress level	$S_1$	$S_2$	$S_3$	
No. of cycles	$n_1$	$n_2$	$n_3$	
No. of cycles to failure	$N_1$	$N_2$	$N_3$	
Incremental damage	$\frac{n_1}{N_1}$	$\frac{n_2}{N_2}$	$\frac{n_3}{N_3}$	...
Cumulative damage	$\frac{n_1}{N_1}$	$\frac{n_1}{N_1} + \frac{n_2}{N_2}$	$\frac{n_1}{N_1} + \frac{n_2}{N_2} + \frac{n_3}{N_3}$	

An NPTEL logo is in the bottom left corner.

Now, according to Palmgren miner hypothesis, what we do is we find out the number of cycles at different stress levels - S 1 is n 1, S 2 is n 2, S 3 is N 3 and so on and so forth, and for this stress level from S N curve, we get the number of cycles to failure. We define incremental damage as the ratio of n 1 by capital N 1. Similarly, incremental damage at stress level n 2 by capital level N 2 and so on and so forth. We define what is known as cumulative damage as the sum of this incremental damage at any given stage. For example, at the end of application of S 2, stress level S 2 for N 2 number of cycles, the accumulated damage would be n 1 by capital N 1 plus n 2 by capital N 2.

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Accumulated damage at the end of m - th cycle

$$\Delta = \sum_{i=1}^m \frac{n_i}{N_i} = \sum_{i=1}^m \frac{n_i S_i^b}{c}$$

Condition for failure

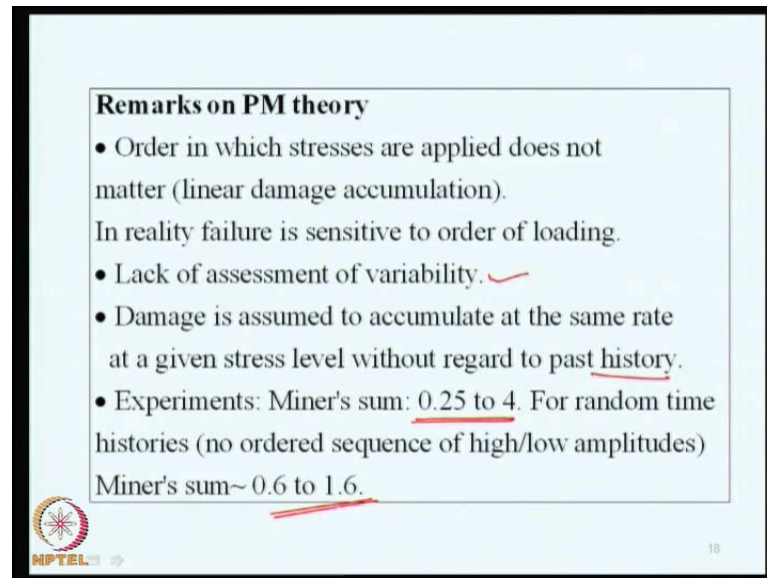
$$\Delta = 1$$

**Safe Limit**

$$\sum_{i=1}^m \frac{n_i S_i^b}{c} \leq 1$$

Now, according to the palmgren miner hypothesis at the end of m th cycle of oscillation, we get the accumulated damage as capital delta is summation i equal to 1 N i by capital n i. Now, if I use the formula for the S N curve, I can write this as N i S i to the power of b by c because capital n i is related to S i and the constant b and c. According to palmgren miner hypothesis, the condition for failure is that this cumulative damage should reach a value of 1. So, consequently the safe limit is i equal to 1 n i S i to the power of b divided by c must be less than or equal to 1. If this miner sum crosses 1, the specimen deemed to have failed according to this theory.

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**Remarks on PM theory**

- Order in which stresses are applied does not matter (linear damage accumulation).  
In reality failure is sensitive to order of loading.
- Lack of assessment of variability. ✓
- Damage is assumed to accumulate at the same rate at a given stress level without regard to past history.
- Experiments: Miner's sum: 0.25 to 4. For random time histories (no ordered sequence of high/low amplitudes) Miner's sum ~ 0.6 to 1.6.

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A few remarks on the Palmgren-Miner theory could be made. In this theory, the order in which stresses are applied does not matter; that means the damage is assumed to take place in a linear manner. In reality, actually failure is sensitive to order of loading. A high stress level followed by low stress level would produce accumulated damage of a different kind than the other way round where a low stress cycle is followed by high stress cycle.

The method as such does not provide any means of assessing the effect of variability in constant  $b$  and  $c$  etcetera, and damage is assumed to accumulate at the same rate at a given stress level without regard to the past history; that means it does not take in to account the memory. In actual experiments, the miner sum at which the failure as occur actually varies over very large range, that is, 0.25 to 4 under harmonic loading, but for random time histories where no ordered sequence of high and low amplitudes exist, the miner sum is reasonably good 0.6 to 1.6.

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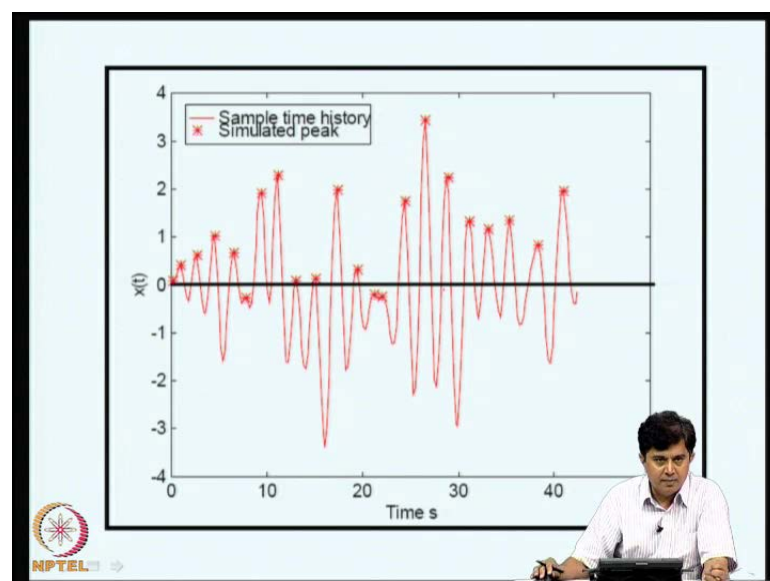
**Extension of PM theory to random stress time histories**

- Assumption: stress time history is a zero mean, narrow band, Gaussian, random process
- There are no "discrete" stress levels

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Now, the question is we are not so much interested in the limitations and scope of palmgren miner hypothesis etcetera. What we would like to consider is how can we extend the palmgren miner hypothesis to random stress time histories. Now, let us assume that the stress time history is a 0 mean narrow band Gaussian random process, and consequently if you consider stress to be a time history, there are no discrete stress levels here, is there not harmonics of different amplitudes following each other. So how do we proceed?

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So, for example, this is a sample of a narrow band time history, time history of a narrow band random process. The notion of cycle is not very evident here. This is a narrow band process, but in a broad band process, as we already seen the notion of a cycle becomes much less apparent.

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$$D(T) = \int_0^T \chi(t) dt$$

$$\chi(t) = \text{rate of accumulation of damage}$$

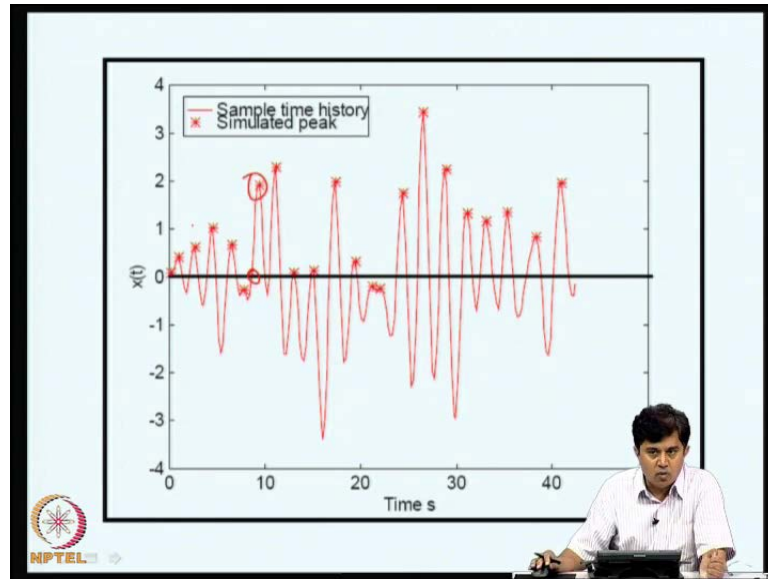
$$\chi(t) = \frac{n(t)S^b(t)}{c} \quad \left[ \text{Fashioned after } D = \frac{n_r S_i^b}{c} \right]$$

Interpretation  
 $S(t)$  = peak magnitude  
 $n(t)$  = rate of peaks  
 = rate of zero crossings for narrow band process

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Here, what we define is function D of T. We define as integral 0 to t chi of T dt - where chi of t is a rate of accumulation of damage. This is, this we write as N of t into S to the power of b of t divided by c and this is fashioned after the formula for accumulation of damage given by n i S i to the power of b divided by c.

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Now, the question is - if  $S$  of  $t$  is a as I said 0 mean narrow band stationary Gaussian random process, how do interpret these quantities? If the stress time history is a random process, how do interpret  $S$  of  $t$   $n$  of  $t$  in this expression? So, what we do is, we take  $S$  of  $t$  to the peak magnitude and  $n$  of  $t$  to be the rate of peaks, and these we have studied already. For a narrow band process, rate of peaks is same as rate of 0 crossings for narrow band processes; that means every time a 0 is crossed, there will be a peak as you see here. For example, there is 0 crossing; there is a peak. So, that is a property of a narrow band process; that is not true for wideband process but I will to start with will limit or discussion to narrow band process.



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$$D(T) = \int_0^T \chi(t) dt$$

$$\chi(t) = \text{rate of accumulation of damage}$$

$$\chi(t) = \frac{n(t)S^b(t)}{c} \quad \left[ \text{Fashioned after } D = \frac{n_i S_i^b}{c} \right]$$

Interpretation  
 $S(t)$  = peak magnitude  
 $n(t)$  = rate of peaks  
 = rate of zero crossings for narrow band processes

Now, our objective is to characterize this function D of capital T, is we are interested in knowing for example, what is probability that d of capital t is less than or equal to 1. That is a question that we would like to answer.

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The joint density function  $p_{Sn}(s, n; t)$  is not available.

**What we know?**

- Peak magnitudes are Rayleigh distributed (approximation for narrow band processes)
- $\langle n \rangle = \frac{\sigma_2}{2\pi\sigma_1}$

Adhoc assumption:  $p_{Sn}(s, n; t) = p_S(s; t) p_n(n; t)$

$$\Rightarrow \langle \chi(t) \rangle = \left\langle \frac{n(t)S^b(t)}{c} \right\rangle = \frac{1}{c} \langle n(t) \rangle \langle S^b(t) \rangle$$

$$= \frac{1}{c} \frac{\sigma_2}{2\pi\sigma_1} \int_0^\infty s^b \frac{s}{\sigma_1^2} \exp\left(-\frac{s^2}{2\sigma_1^2}\right) ds$$

Now, the joint density a function of S and N is actually not available. So, what we actually know? We know the peak magnitudes are Rayleigh distributed for narrow band processes and expected value of n is given by sigma 2 divided by 2 pi by 2 pi into sigma

1 - where sigma 2 is the standard deviation of the derivative of the process and sigma 1 is the standard deviation of the palmgren process.

Now, we make in Adhoc assumption that S and N are independent. So, we assume that joint density function PSN s, n is product of the marginal density functions. So, based on that, we find the expected value of chi of t which is the rate at which the damage is accumulating, and since we know this expression and we know this probability density function, we can write for this expected value this form. This is Rayleigh density function. So, consequently this will be the expected value of S to the power of b, and this is, this quantity is the actual expected value of n of t, which is again known from study of low level crossings and 0 crossings of random process.

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Recall:

$$\Gamma(\nu) = 2 \int_0^{\infty} y^{2\nu-1} \exp(-y^2) dy \quad \text{gamma fun}$$

$$\Rightarrow$$

$$\langle \chi(t) \rangle = \frac{1}{c} \frac{\sigma_2}{2\pi\sigma_1} \Gamma\left(\frac{b+2}{2}\right) (\sigma_1\sqrt{2})^b$$

$$\langle D(T) \rangle = \int_0^T \langle \chi(t) \rangle dt = \frac{T}{c} \frac{\sigma_2}{2\pi\sigma_1} \Gamma\left(\frac{b+2}{2}\right) (\sigma_1\sqrt{2})^b$$

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Now, we recall that the integral of this type 0 to infinity y to the power of 2 nu u minus 1 exponential minus y square dy is actually given by the gamma function. Now, consequently we can express the expected value of chi of t in terms of the gamma function and this is what we get. So, at least we are able to proceed and get the certain Adhoc assumptions the average rate at which fatigue damage is accumulating in terms of the standard deviation of the parent process, standard deviation of the derivative process and the material concuss c and b.

Now, we can return to the problem of finding expected value of  $d$  of  $t$ . Now, if process is stationary, we see that  $\sigma_2$  and  $\sigma_1$  are independent of time. So, they can be pulled out and this  $0$  to  $t$   $d t$  will become this. So, we have this expression for  $d$  of  $t$ . That is average of the accumulated damage at time instance capital  $T$ .

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The slide contains the following text and equations:

$$\langle D(T) \rangle = \frac{T}{c} \frac{\sigma_2}{2\pi\sigma_1} \Gamma\left(\frac{b+2}{2}\right) (\sigma_1\sqrt{2})^b$$

We could visualize a  $T^*$  such that

$$\langle D(T) \rangle = 1 \Rightarrow T^* = \frac{1}{\langle \chi(t) \rangle} //$$

$T^*$  can be taken as the approximate mean fatigue life.

The slide also features the NPTEL logo in the bottom left corner and a small image of a person sitting at a desk in the bottom right corner.


Now, we could visualize a  $T^*$  such that these expected value of  $D$  of  $T$  becomes 1. Now, if you do that, that  $T^*$  will be 1 by expected value of  $\chi$  of  $t$ . So, to a first approximation, we can take  $T^*$  to be the mean fatigue life. So, if you know the random stress time history and you know its prospect density, you can derive the variance of the process and its derivative process, and if use this formulary, you have a handle now that, you can at least, you are at least in a position to evaluate the mean fatigue life. This is for a stationary narrow band Gaussian random process.

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**Remarks**

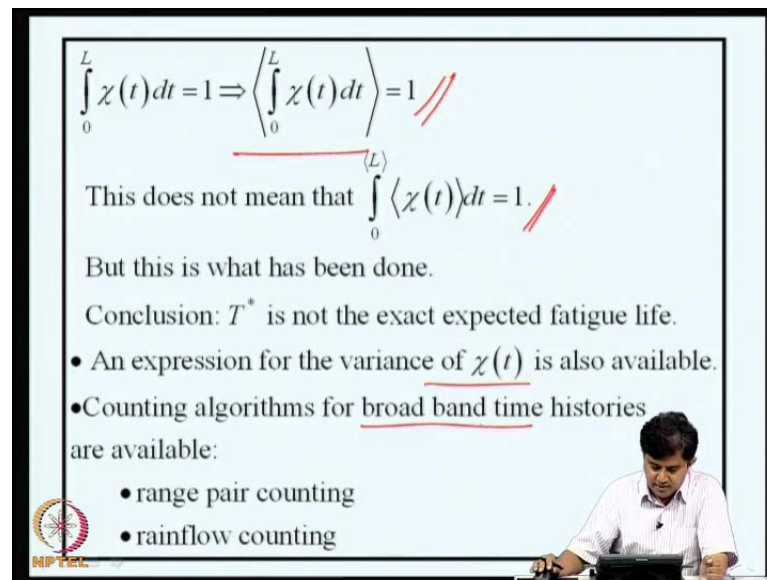
- b and c have been assumed to be deterministic constants. They can be treated as random variables in which case the above expectation has to be interpreted as a conditional expectation.
- Is T<sup>\*</sup> indeed the expected fatigue life?  
Let *L* be the life time. It is a random variable.

Condition for failure is  $\int_0^L \chi(t) dt = 1 \Rightarrow \left\langle \int_0^L \chi(t) dt \right\rangle = 1.$

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Now, a few remarks are in order. In this particular derivation, we have taken the constant *b* and *c* to be deterministic, but if you happen to know the joint probability density function of *b* and *c*, we could incorporate that into the analysis. So, what the expectation that we have got can be interpreted as a conditional expectation and the unconditional mean can be evaluated by an integration with respect to the conditional probability density function to the joint probability density function of *b* and *c*. Is worth considering the equation is *T* star indeed the expected fatigue life. Now, we will this needs to be consider carefully. So, for that purpose, we will consider capital *l* to be the life time. Life time is a random variable. So, condition for failure that is a definition of capital *l* is 0 to *l*  $\chi$  of *t* *d t* is 1; that means the expected value of this integral 0 to *l*  $\chi$  of *t* *d t* is 1.

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$$\int_0^L \chi(t) dt = 1 \Rightarrow \left\langle \int_0^L \chi(t) dt \right\rangle = 1 //$$


This does not mean that  $\int_0^L \langle \chi(t) \rangle dt = 1. //$

But this is what has been done.

Conclusion:  $T^*$  is not the exact expected fatigue life.

- An expression for the variance of  $\chi(t)$  is also available.
- Counting algorithms for broad band time histories are available:
  - range pair counting
  - rainflow counting





Now, this is a condition that we actually need to analyze, but what we have done is we have placed this integration, the random quantities in this expression is  $L$  as well as  $\chi$  of  $t$  and they are mutually dependent. This integral obviously not equal to this. This is 0 to mean of  $L$ . Then mean of the integrand  $dt$  is not the, actually the condition that we are looking for all those. This is what we seen to have utilized. So, the conclusion is  $T^*$ , is not the exact expected fatigue life, but if first approximation, we could take it as an approximate, an acceptable estimate, and if you want to really evaluate this integral in exactly, this is the, we do not have adequate information on the uncertainties here. Therefore, we will not able to proceed anyway.

Now, we have derived now the expression for expected value of  $\chi$  of  $t$ . With little effort, we can also derive the variance of  $\chi$  of  $t$  and we can also derive variance of capital  $d$  of  $t$ , and we have based our assumption on the fact that where assuming that the stress time is to the narrow band process. If the process is broad banded, then the notion of a cycle and amplitude becomes less apparent as I have already said, but there are algorithms for counting cycles and some of the algorithms are range per counting range flow counting etcetera. They are available one could also deal with broad band time histories.

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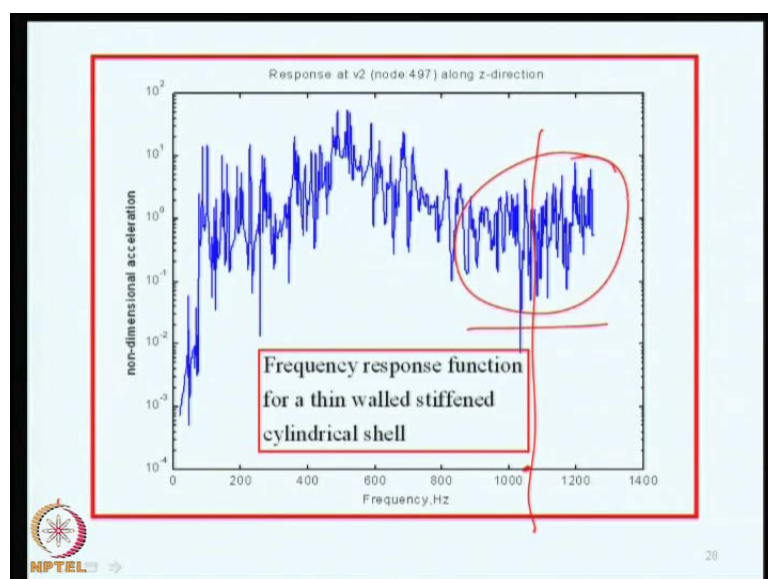
**Analysis of Vibration in high frequency regime using random vibration principles**

We consider that a structure is vibrating in the *high frequency regime* if its response at any frequency consists of significant contributions from a large number of modes

The slide features the NPTEL logo in the bottom left corner and a photograph of a man sitting at a desk with a laptop in the bottom right corner.

So, what have done in this brief presentation is to illustrate how properties of level crossing and peak distribution etcetera could be utilized to get a theoretical estimate of average, the expected life due to when failure is defined with respective fatigue. Now, we will move on to another application. This is analysis of vibration in high frequency regime and how we can use random vibration principles for this. Now, what is high frequency vibration? We consider that a structure is vibrating in the high frequency regime. If it is response at any frequency, consists of significant contributions from a large number of modes.

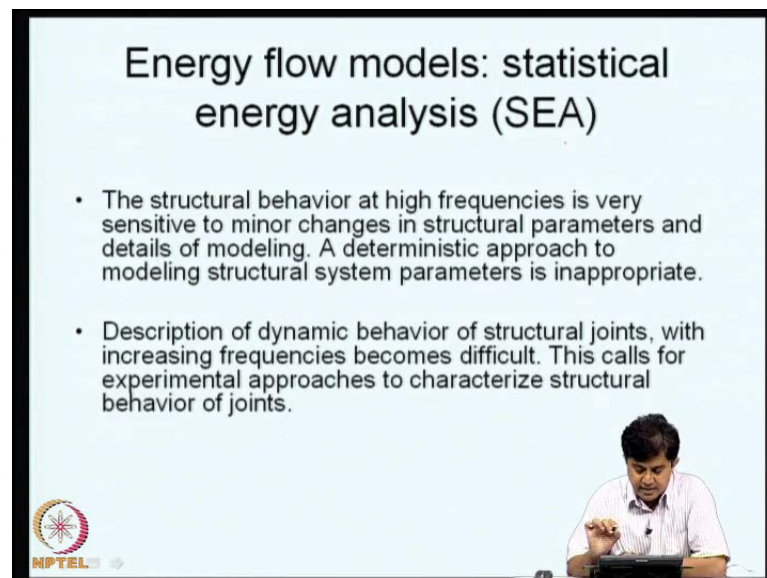
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Now, what are the basic problems in such, such, basic difficulties in such problems? For purpose of illustration, I have shown here the transfer function of a, frequency response function of a thin walled stiffened cylindrical shell, and as you can see here, there are several peaks which are closely space. We know that in frequency response function, peaks occur wherever there is a (( )) frequency and their variation is also governed by value of mode steps at point of driving at point of measurement.

So, if we consider segments like this, it is very clear that there are large number of natural frequencies that are crowded in this frequency range. So, if you were to drive the system harmonically at a single frequency, it is expected that to compute the response, we need to consider contributions from several neighboring modes. In that sense, we call this type of oscillations is high frequency oscillations. This is not true for example in the case of say earthquake response of tall buildings, but whereas the in acoustic response of missile shells or aircraft wings etcetera, there will be large number of natural frequencies or in the frequency range that we are interested. I will clarify the nature of the problem as we go long.

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The slide features a light blue background with a black border. At the top center, the title "Energy flow models: statistical energy analysis (SEA)" is displayed in a bold, black, sans-serif font. Below the title, there are two bullet points, each starting with a black dot. The first bullet point discusses the sensitivity of structural behavior at high frequencies to minor changes in parameters and modeling details, noting that a deterministic approach is inappropriate. The second bullet point discusses the difficulty of describing dynamic behavior of structural joints at increasing frequencies, suggesting experimental approaches. In the bottom right corner, there is a small inset image of a man in a white shirt sitting at a desk with a laptop. In the bottom left corner, the NPTEL logo is visible, consisting of a circular emblem with a star-like pattern and the text "NPTEL" below it.

Now, there is one approach to studies such high frequency vibration and that is known as energy flow models, and one of the techniques are one of the methodology there is what is known as statistical energy analysis, is a frame work for analyzing high frequency vibration analysis. Now, the structural behavioral at high frequencies is very sensitive to

minor changes in structural parameters and details of modeling. A deterministic approach to modeling structural system parameters is inappropriate; that means if a structure is undergoing high frequency oscillations, a slight change in say boundary conditions or some of the parameters of the problem will dramatically alter the behavior of the structure.

So, the essence here is that we need to consider this extreme sensitivity of the response with respective changes in system parameters. Description of dynamic behavior of structural joints with increasing frequencies become difficult. This calls for experimental approaches to characterize structural behavior of joints. So, in typical structure like automobile vehicle or aircraft structure, there will be several joints, client joints, point joints, etcetera and their behavior at high frequencies becomes quiet complicated, and if use methods like finite element method modeling, the flexibility of the joints becomes a very complicated issue as you go very higher up in the frequencies.



So, the, this statistical energy analysis procedures essentially take in to account these difficult issues. First thing, it does is it treats the vibrating system as being randomly parameter; that means a natural frequency is more shapes of this structure is treated as being random in nature. The word statistical in statistical energy analysis refers to uncertainties in specifying the system parameters. The excitations could be random or could be deterministic. So, the stochastic nature in these problems come originate due to randomness in system properties.



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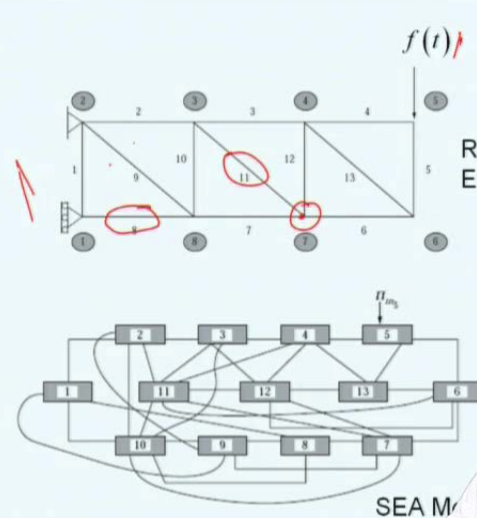
### Energy flow models: statistical energy analysis (SEA)

- Detailed characterization of structural response in terms of spatio-temporal variations of displacement / stress/strain fields becomes unwieldy. Macro-level descriptions that involve space-time-frequency averaged response quantities, such as, vibration energy content in spatial domains, within a structure, may be adequate.
- Thus, method of analysis aimed at establishing spatial distribution of vibration energy stored in the structure become appropriate in analysis of high frequency vibrations.



In such problems, detailed characterization of structural response in terms of spatiotemporal variations of displacement stress and strain fields become unwieldy. Macro level description that involves space time frequency averaged response quantities such as vibration energy content in spatial domains within a structure may be adequate. So, the method of analysis is aimed at establishing spatial distribution of vibration energy stored in the structure as a function of frequency.

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$f(t)$

Randomly Excited truss

$m_5$

SEA M



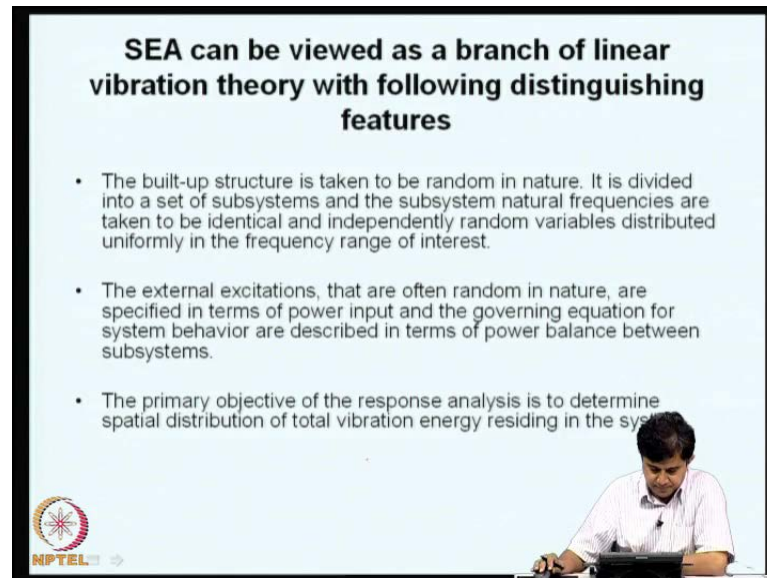
So, I will just illustrate what, what the, what that means, what I said. Consider a simple truss here which is carrying a force  $f$  of  $t$ . Let us assume that the frequency range of  $f$  of  $t$  spans several decades of frequencies. It may go up to a few tens of hertz to couple of thousands of hertz. We could use finite element method to analyze this problem, but then, if we need to compute the natural frequencies of this structure over a frequency range of say 2 kilo hertz, there could be several may be hundreds or couple of hundreds of natural frequencies which could lie in that frequency range.

Now, if we indeed perform, that would mean that we need to model this structure in greater detail. We need to select element which are quite small in size. We should have large number of such elements that intend requires, that we should be able to know the property of this structure at smaller levels, smaller spatial levels and that may not be always possible. Similarly, the question on how does the joint behave as a frequency increases becomes crucial.

So, any slight change in system parameters here can dramatically alter the behavior of the system. So, therefore, if you are interested in for example displacement stress or strain at any given point in the structure, we end up doing huge amount of calculations, but that type of information may not be needed to take decisions in this types of problems. What do we may like to know is what is the total energy that resides in this member or how does a total energy gets spatially distributed in the system.

For that purpose, what we do is we divide the truss into a set of subsystems. Each one is an energy carrying unit, and we would like to now consider the problem of this force into the velocity here will be a power input to the system. That power should be stored at various points in the structure and they, which are they more energetic members, and if that level of energy is high, how do I reduce it? What are the vibration paths that take this energy two different parts in the structure? This type of questions are asked in, in, statistical energy analysis.

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**SEA can be viewed as a branch of linear vibration theory with following distinguishing features**

- The built-up structure is taken to be random in nature. It is divided into a set of subsystems and the subsystem natural frequencies are taken to be identical and independently random variables distributed uniformly in the frequency range of interest.
- The external excitations, that are often random in nature, are specified in terms of power input and the governing equation for system behavior are described in terms of power balance between subsystems.
- The primary objective of the response analysis is to determine spatial distribution of total vibration energy residing in the system.

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So, statistical energy analysis can be viewed as a branch of linear vibration theory with some of the following characteristic features. Here, the built up structure is taken to be random in nature. It is divided into a set of subsystems and the subsystem natural frequencies are taken to be identical and independent random variables distributed uniformly in the frequency range of interest. That could mean the natural frequencies of the subsystems are taken to constitute Poisson points on the frequency axis; that means natural frequencies are like number of natural frequencies is a Poisson random variables. The external excitation that are often random in nature are specified in terms of power input, and the governing equation for system behavior are described in terms of power balance between subsystems. We do not write the equilibrium equation the way we do in finite element method. We simply keep track of energy balance. I will come to some details.

The primary objective of the response analysis is to determine the spatial distribution of total vibration energy residing in the system. It is not to obtain, for example, time histories of stresses, strain and displacement at every point on the structure. Here, it is a macro level description where certain spatial extends are identified as subsystems, and what happens within that spatial extend in terms of a single number is what is being sort.

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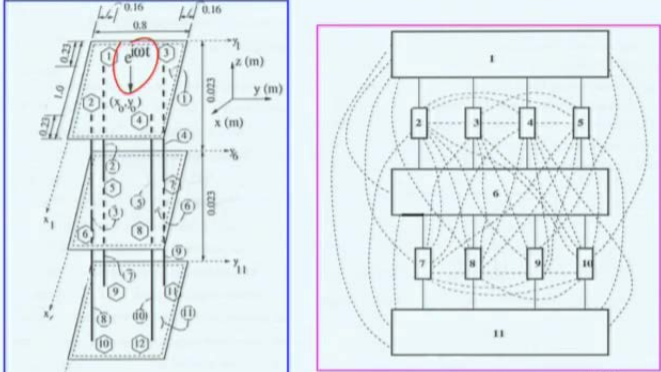
### References

- R H Lyon and R G DeJong, 1995, Theory and application of statistical energy analysis, 2nd Edition, Butterworth-Heinmann, MA.
- J Woodhouse, 1981, An introduction to statistical energy analysis of structural vibration, Applied Acoustics, 14, 455-469.
- C S Manohar and R A Ibrahim, 1999, Progress in structural dynamics with stochastic parameter variations, 1987-98, Applied Mechanics Reviews, 52(5), 177-197.





Here are some references - the book by the Lyon and DeJong theory and application of statistical energy analysis. There is an introductory paper, and a review paper which provides some background to this subject.

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Schematic diagram of the plate assembly; dimensions not to scale

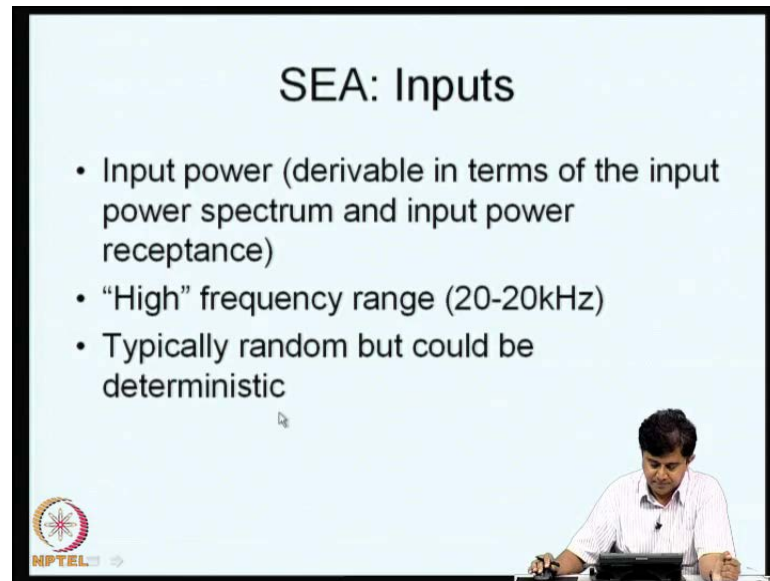
SEA Model



Here, there is another example where there are a stack of three plates. This could be a typical situation in solar panel arrays in satellite. This  $e^{i\omega t}$  is a forcing function that acts on this and this frequency can cover several, you know, it could run from twenty to say two kilo hertz. What we are interested in knowing is under this kind

of excitations, how does vibration energy reside in different parts? So, we could model this in terms of subsystems as shown here 1, 6 and 11 correspond to these three plates. This, 1, 6 and 7, 1, 6 and 11 correspond to these three plates and 2 3 4 5 etcetera, 7 8 9 10 are this coupling members. We are not interested. It may again emphasis in finding stresses strain and displacement in various points on the structure.

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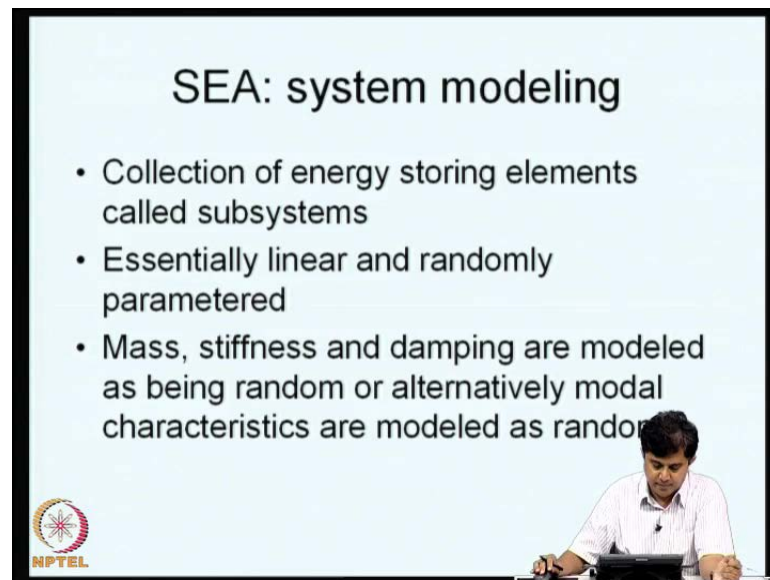
The slide is titled "SEA: Inputs" and contains the following bullet points:

- Input power (derivable in terms of the input power spectrum and input power receptance)
- “High” frequency range (20-20kHz)
- Typically random but could be deterministic

In the bottom left corner, there is a circular logo with a star-like pattern and the text "NPTEL" below it. In the bottom right corner, there is a small inset image of a man in a white shirt sitting at a desk with a laptop, looking down at the screen.

Now, how do we specify inputs in statistical energy analysis? We do not know time history of the force but we would like to provide the input power. This input power is derivable in terms of the input power spectrum and the input power receptance; that means the force into the velocity, applied force into the velocity in the direction of the force is a power. Force into displacement is a work done and for that work done for a unit time is the input power, and in terms of a spectrum if you want, you have to do certain calculation. Now, we consider inputs to be in the high frequency range say 20 to 20 k kilo hertz, these excitations could be random or they could be deterministic also. It is not important that the randomness resides in the excitation. The randomness is in the system parameters.

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The slide is titled "SEA: system modeling" and contains three bullet points. In the bottom left corner, there is a circular logo with a star and the text "NPTEL". In the bottom right corner, there is a small inset image of a man sitting at a desk with a laptop, looking at the screen.

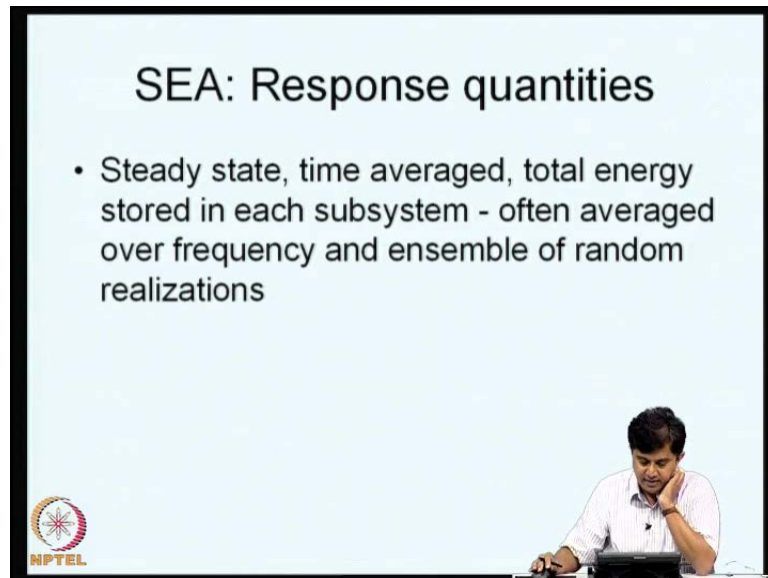
### SEA: system modeling

- Collection of energy storing elements called subsystems
- Essentially linear and randomly parametered
- Mass, stiffness and damping are modeled as being random or alternatively modal characteristics are modeled as random

How do we model a system? We model a system as collection of energy storing elements called subsystems. They are not like finite element elements but they are much more macro level description. They involve much, you know, macro level description of the system behavior. We focus only on linear and randomly parametered systems. The behavior is linear but the system is randomly parametered.

This randomness in system parameters could be introduced at the level of definition of mass stiffness and damping as being random or alternatively we can directly characterize the natural frequencies and mode shapes of the subsystems as being random. We need not have to specify mass stiffness and damping as random but since solution is being obtained in model domain. The model parameters like natural frequency, mode shapes participation factor modal damping etcetera could be taken as random.

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**SEA: Response quantities**

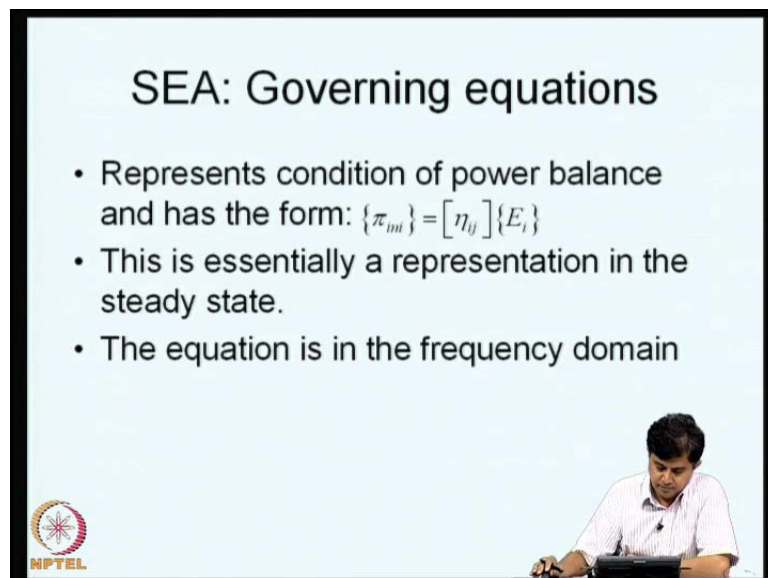
- Steady state, time averaged, total energy stored in each subsystem - often averaged over frequency and ensemble of random realizations

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And what are the response quantities we look for? We look for steady state, time averaged, total energy stored in each subsystem often averaged over frequency and ensemble of random realizations; that means we are looking for grossly averaged features. We are not looking into details. It is average over time, average over space, average over frequency band and it gives a broad idea about different levels of energy. They stay at, they decide at different levels.

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**SEA: Governing equations**

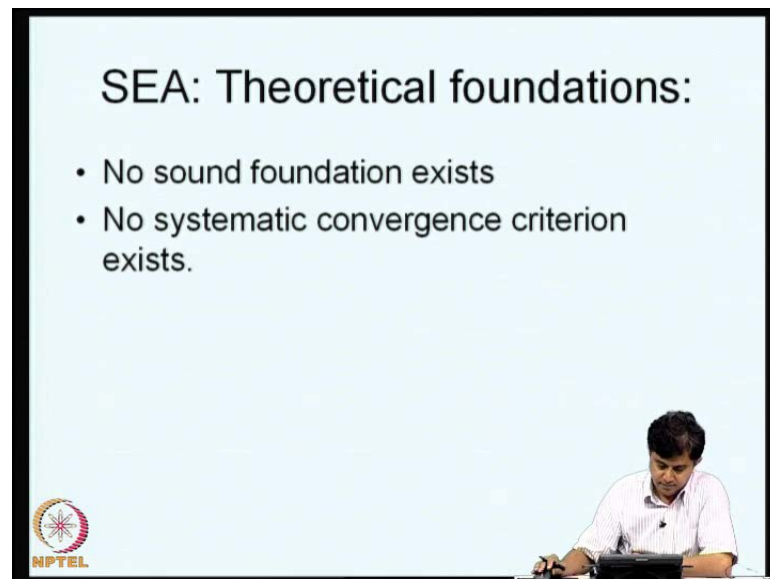
- Represents condition of power balance and has the form:  $\{\pi_m\} = [\eta_{ij}] \{E_i\}$
- This is essentially a representation in the steady state.
- The equation is in the frequency domain

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How do we derive the governing equation? What are the governing equations in SEA? The governing equation in SEA represents condition of power balance and it has the form the vector of power input is equal to a matrix multiplied by energy at different energy residing in different subsystems. This matrix is known as coupling loss factor matrix. I will talk about this shortly. This is essentially a representation in the steady state. We do not normally consider transient behavior using SEA, and the whole analysis is done in frequency domain.

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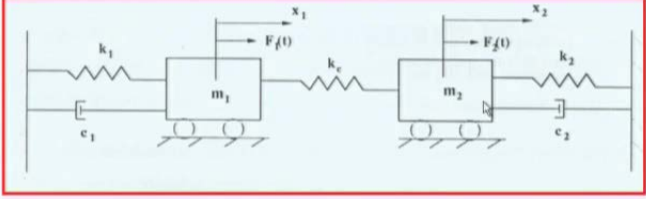


Now, we can ask are there any theoretical foundations for this procedures? Actually there is no such regress theoretical foundation. There is no, for example, a variational principle from which we can derive SEA equations nor there is any systematic proof that as we increase the number of subsystems in SEA formulation. The answer would converge. So, it is not like a method like finite element method but it is a heuristic procedure which is found to work well when other methods do not perform.





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**Two coupled oscillators: the case of exact results in SEA**

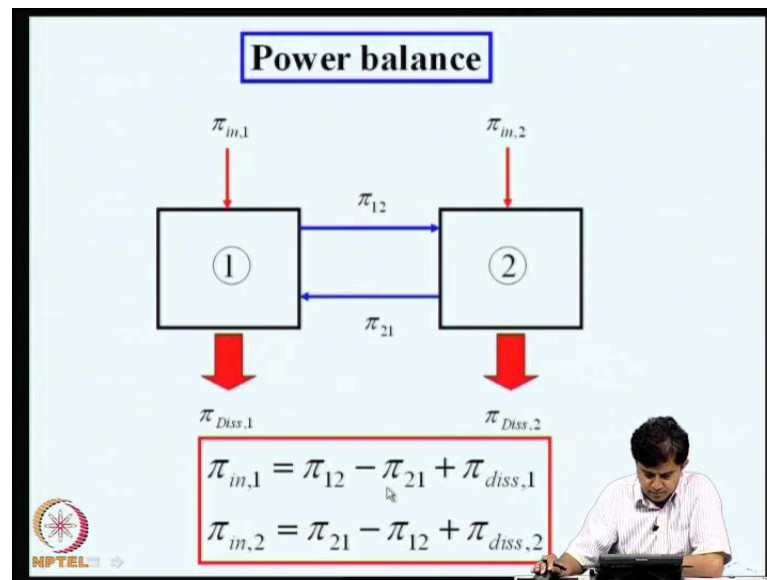


$F_1(t)$  &  $F_2(t)$ : zero mean stationary Gaussian mutually independent random processes (broad banded; ideal white noise processes)



Now, the, having said that there is no theoretical basis but there exist an idealize situation where statistical energy analysis basic results of statistical energy analysis can be shown to hold good. So, what is that system? This is simple system where two oscillator which are couples, a coupling spring. The two oscillators are driven by  $F_1(t)$  and  $F_2(t)$ . These are taken to be independent Gaussian White Noise Processes. Now, the question that would like to answer is this  $F_1(t)$  will do some work on this displacement. Therefore, there will be a power input here and there will be a power input here and how does vibration energy gets shared between this subsystem and this subsystem.

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So, what we do is we model the problem using two subsystems - this is one oscillator this is second oscillator there is a coupling  $k_c$ , and please note that the coupling element do not have, does not have damping. It is a conservative coupling. So, what happens? There is a power input to the first system through  $F_1(t)$  and part of the power is dissipated through damping and part of that flows to the, energy flows to the second subsystem.

In the second system because  $F_2(t)$ , there is some power input, and part of it is dissipated and part of it flows to a subsystem one. So, now, we set up this simple equation that  $\pi_{in,1}$  is equal to  $\pi_{12}$  minus  $\pi_{21}$  plus  $\pi_{diss,1}$ . So,  $\pi_{in,1}$  must equal to this. Similarly, what comes in to second system?  $\pi_{in,2}$  plus  $\pi_{12}$ , and what goes out?  $\pi_{diss,2}$  and  $\pi_{21}$ . So,  $\pi_{in,2}$  must equal to  $\pi_{21}$  minus  $\pi_{12}$  plus  $\pi_{diss,2}$ . So, this is, there is no approximation here. This is an exact.

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Equations of motion

$$m_1 \ddot{y}_1 + c_1 \dot{y}_1 + k_1 y_1 + k_c (y_1 - y_2) = F_1(t)$$
$$m_2 \ddot{y}_2 + c_2 \dot{y}_2 + k_2 y_2 + k_c (y_2 - y_1) = F_2(t)$$
$$\langle F_1(t) \rangle = 0; \langle F_1(t) F_1(t + \tau) \rangle = I_1 \delta(\tau)$$
$$\langle F_2(t) \rangle = 0; \langle F_2(t) F_2(t + \tau) \rangle = I_2 \delta(\tau)$$
$$\langle F_1(t) F_2(t + \tau) \rangle = 0$$

The equation can be recast as

$$m_1 \ddot{y}_1 + c_1 \dot{y}_1 + (k_1 + k_c) y_1 = F_1(t) + k_c y_2(t)$$
$$m_2 \ddot{y}_2 + c_2 \dot{y}_2 + (k_2 + k_c) y_2 = F_2(t) + k_c y_1(t)$$

As  $t \rightarrow \infty$ , the system reaches steady state



Now, let us try to evaluate the expressions for this input powers and maybe I will take expected values and see what happens. So, to do that, we will write the equations of motion. Now, the equation of motion consist this simple to derive the 2 degree approximation, 2 degree freedom model. It guess couple through the spring  $k_c$  and this is reasonably straight forward, and we are assuming that  $F_1(t)$  is 0 mean and it is a white noise with strength  $I_1$  and  $F_2(t)$  also is a white noise with strength  $I_2$  is 0 mean, and we assume that the uncorrelated; that means they are independent because we are assuming Gaussian White Noise models. The equation can be recast as in this form where we are taking the force acting on the first system through the coupler on the right hand side.

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**Digress**

$$m\ddot{x} + c\dot{x} + kx = f(t)$$

$$\langle f(t) \rangle = 0; \langle f(t)f(t+\tau) \rangle = \delta(\tau)$$

Consider  $t \rightarrow \infty$


$$S_{xf}(\omega) = \lim_{T \rightarrow \infty} \frac{1}{T} \langle X_T(\omega) F_T^*(\omega) \rangle$$

$$= \lim_{T \rightarrow \infty} \frac{1}{T} \langle H(\omega) F_T(\omega) F_T^*(\omega) \rangle$$

$$= H(\omega) S_{FF}(\omega) = H(\omega) S_0$$

$$R_{xf}(\tau) = \frac{1}{2\pi} \int_{-\infty}^{\infty} H(\omega) S_0 \exp(-i\omega\tau) d\omega$$

$$\Rightarrow R_{xf}(\tau) = \frac{S_0 h(\tau)}{2\pi} \Rightarrow R_{xf}(0) = \frac{S_0 h(0)}{2\pi} =$$



So, we assume the system since the system is damped, we assume we assume that as  $c$  tends to infinity the system reaches steady state. The input stationary; system is damped. Now, we will digress for a moment. If you consider a single degree freedom system under a white noise excitation in the steady state, if you want to find out the cross power spectral density function between displacement and applied force, this can be derived in terms of this well-known expression, and the Fourier transform of the response is related to the Fourier transform of the input through the system transfer function, and we get this  $S_{xf}$  of  $\omega$  to be  $H$  of  $\omega$  into  $S_{FF}$  of  $\omega$ . Since excitation is white noise, I get this  $H$  of  $\omega$  into  $S_0$ .

Now, it is a cross covariant is a Fourier transform of this. So, I get this expression. Now, a  $\tau$  equal to 0. I will get  $R_{xf}(0)$ .  $R_{xf}(0)$  can be obtained by putting  $\tau$  equal to 0 but you can see that the integrand is nothing but Fourier Transform of the impulse response function. So,  $R_{xf}(\tau)$  is  $S_0 H(\tau)$  by  $2\pi$ . Now, in this if we put  $\tau$  equal to 0, I get  $R_{xf}(0)$  which is  $S_0 h(0)$  by  $2\pi$ . Now, we know definition of  $h(0)$  is 0. Therefore,  $r_{xf}(0)$  is 0. So, that result I will be using so that this digression to make sure that, that is, registers on your mind.

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**Recall**  
 If  $X(t)$  and  $Y(t)$  are jointly stationary, we have shown that

$$\left\langle \frac{d^n X(t+\tau)}{dt^n} \frac{d^m Y(t)}{dt^m} \right\rangle = (-1)^m \frac{d^{n+m} R_{XY}(\tau)}{d\tau^{n+m}}$$

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Now, we would also need another result. If  $X$  of  $t$  and  $Y$  of  $t$  are jointly stationary, the expected value of the product of  $n$  th derivative and  $m$  th derivative of  $X$  and  $Y$  as shown here is given by this. This we have derive when we introduce the basic motions of mean square derivative for random processes.

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$$m_1 \ddot{y}_1 + c_1 \dot{y}_1 + (k_1 + k_c) y_1 = F_1(t) + k_c y_2(t)$$

$$m_2 \ddot{y}_2 + c_2 \dot{y}_2 + (k_2 + k_c) y_2 = F_2(t) + k_c y_1(t)$$

Consider  $t \rightarrow \infty$ .

It can be verified that (Exercise)

$$\langle F_1(t) y_1(t) \rangle = 0; \langle F_1(t) y_2(t) \rangle = 0; \langle F_2(t) y_1(t) \rangle = 0; \langle F_2(t) y_2(t) \rangle = 0$$

$$\langle \dot{y}_1(t) \dot{y}_1(t) \rangle = 0; \langle \dot{y}_2(t) \dot{y}_2(t) \rangle = 0; \langle \dot{y}_1(t) \dot{y}_2(t) \rangle = 0; \langle \dot{y}_2(t) \dot{y}_1(t) \rangle = 0$$

$$\langle y_1(t) \ddot{y}_1(t) \rangle = -\langle \dot{y}_1^2(t) \rangle; \langle y_2(t) \ddot{y}_2(t) \rangle = -\langle \dot{y}_2^2(t) \rangle;$$

$$\langle \ddot{y}_1(t) \dot{y}_2(t) \rangle = -\langle \dot{y}_1(t) \ddot{y}_2(t) \rangle; \langle \dot{y}_1(t) \ddot{y}_2(t) \rangle = -\langle \dot{y}_1(t) \dot{y}_2(t) \rangle$$

$$\langle \ddot{y}_1(t) y_2(t) \rangle = -\langle \dot{y}_1(t) \dot{y}_2(t) \rangle; \langle \dot{y}_1(t) \ddot{y}_2(t) \rangle = -\langle \dot{y}_1(t) \dot{y}_2(t) \rangle$$

$$\langle F_1(t) \dot{y}_2(t) \rangle = 0; \langle F_2(t) \dot{y}_1(t) \rangle = 0;$$

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Now, we can consider certain expectations  $F_1$  of  $t$  into  $y_1$  of  $t$  is 0 using the logic that I just not described. We can also show that sum of these are 0, and since process is stationary, the process and its derivative at same time are uncorrelated. So, again, that

could mean these expectations are 0, and expected value of  $y_1$  into  $y_1$  double dot using this formula can be shown to be given by this, and we have several other expectation that we need when work this equations and I leave it on an exercise to verify each of these statements. These are already covered in the lectures. So, it is a good time to check you are understanding by showing that each one of these statements are true.

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We are interested in

$$\langle \pi_{1,m} \rangle = \langle \underline{F_1(t)} \dot{y}_1(t) \rangle; \langle \pi_{2,m} \rangle = \langle \underline{F_2(t)} \dot{y}_2(t) \rangle$$

$$\langle \pi_{1,diss} \rangle = \underline{c_1} \langle \dot{y}_1^2 \rangle; \langle \pi_{2,diss} \rangle = \underline{c_2} \langle \dot{y}_2^2 \rangle$$

$$\langle \pi_{12} \rangle = k_c \langle y_1 \dot{y}_2 \rangle; \langle \pi_{21} \rangle = k_c \langle y_2 \dot{y}_1 \rangle$$

Since  $\langle y_1(t) \dot{y}_2(t) \rangle = -\langle \dot{y}_1(t) y_2(t) \rangle$  it follows

$$\langle \pi_{12} \rangle = -\langle \pi_{21} \rangle //$$

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Now, what is that we are interested in? We are interested in the average power input, that is, expected value of  $\pi_1$  in. What is power input to the first mass that is applied force  $F_1$  of  $t$  into  $dy_1$  would give the work done and into  $dy_1$  by  $dt$  would give the work done for unit time. So,  $\pi_1$  in expected value nothing but expected value of  $F_1$  into  $y_1$  dot. Similarly,  $\pi_2$  in will be  $f_2$  into  $y_2$  dot. What is power dissipated it is  $c_1 y_1$  dot square;  $\pi_2$  dissipated is this. What is  $\pi_{12}$ ?  $\pi_{12}$  is,  $\pi_{12}$  is the force in the spring acting on velocity of this. So, that would, you can show that, that would be given by  $k_c$  into  $y_1$  into  $y_2$  dot and  $\pi_{21}$  will be  $k_c y_2$  into  $y_1$  dot.

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$$\begin{pmatrix} \langle F_1 \dot{y}_1 \rangle \\ \langle F_2 \dot{y}_2 \rangle \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{pmatrix} = \begin{bmatrix} c_1 & 0 & 0 & 0 & 0 & 0 & 0 & k_c & 0 \\ 0 & c_2 & 0 & 0 & 0 & 0 & 0 & -k_c & 0 \\ 0 & 0 & 0 & 0 & 0 & 0 & c_1 & k_1 + k_c & m_1 \\ 0 & 0 & 0 & 0 & 0 & 0 & c_2 & -k_2 - k_c & m_2 \\ -m_1 & 0 & k_1 + k_c & 0 & -k_c & 0 & 0 & 0 & 0 \\ 0 & -m_2 & 0 & k_2 + k_c & -k_c & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & -k_c & k_1 + k_c & -m_1 & -c_1 & 0 & 0 \\ 0 & 0 & -k_c & 0 & k_2 + k_c & -m_2 & c_2 & 0 & 0 \end{bmatrix} \begin{pmatrix} \langle \dot{y}_1^2 \rangle \\ \langle \dot{y}_2^2 \rangle \\ \langle y_1^2 \rangle \\ \langle y_2^2 \rangle \\ \langle y_1 y_2 \rangle \\ \langle \dot{y}_1 \dot{y}_2 \rangle \\ \langle y_1 \dot{y}_2 \rangle \\ \langle \dot{y}_1 y_2 \rangle \end{pmatrix}$$

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First two rows:

$$c_1 \langle \dot{y}_1^2 \rangle + k_c \langle y_1 \dot{y}_2 \rangle = \langle F_1(t) \dot{y}_1(t) \rangle$$

$$c_2 \langle \dot{y}_2^2 \rangle - k_c \langle y_2 \dot{y}_1 \rangle = \langle F_2(t) \dot{y}_2(t) \rangle$$

$$\Rightarrow$$

$$\langle \pi_{1,diss} \rangle + \langle \pi_{12} \rangle = \langle \pi_{1,in} \rangle$$

$$\langle \pi_{2,diss} \rangle - \langle \pi_{12} \rangle = \langle \pi_{2,in} \rangle$$

Law of conservation of energy

Now, if since  $y_1$  of  $t$  and  $y_2$  dot of  $t$  is 0 and it not 0, it is equal to minus  $y_1$  dot  $y_2$   $t$  follows that  $\pi_{12}$  is equal to minus  $\pi_{21}$ . Now, we can recast all these equations in a matrix form. So, on the left side, I have  $\pi_{1,in}$ ; here,  $\pi_{2,in}$ , and all other expected values I mean I am writing, I mean you have to recast that set of equations in this form we get this equation. This is a straight forward representation of the equation that we consider just now.

Now, in this, we will now consider some of the rows. Suppose if you consider the first two rows, I get  $c_1$  into  $y_1$  dot square expected value plus  $k_c$  into  $y_1 y_2$  dot is equal to  $F_1$  of  $t$  into  $y_1$  dot. Similarly,  $c_2 y_2$  dot minus  $k_c$  into  $y_2 y_1$  dot is this. These two equations are nothing but the statement of law of conservation energy, that, is  $\pi_1$  diss plus  $\pi_1^2$  is  $\pi_1$  in  $\pi_2$  diss minus  $\pi_1^2$  is  $\pi_2$  in. So, things gel-well, there is no problem.

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Rows 5 and 6  $\Rightarrow$

$$-m_1 \langle \dot{y}_1^2 \rangle + (k_1 + k_c) \langle y_1^2 \rangle - k_c \langle y_1 y_2 \rangle = 0$$

$$-m_2 \langle \dot{y}_2^2 \rangle + (k_1 + k_c) \langle y_2^2 \rangle - k_c \langle y_1 y_2 \rangle = 0$$

$\Rightarrow$

$$-\langle KE_1 \rangle + \langle PE_1 \rangle = 2k_c \langle y_1 y_2 \rangle$$

$$-\langle KE_2 \rangle + \langle PE_2 \rangle = 2k_c \langle y_1 y_2 \rangle$$

Remark

In the absence of coupling ( $k_c = 0$ ) we get

$$\langle KE_1 \rangle = \langle PE_1 \rangle; \langle KE_2 \rangle = \langle PE_2 \rangle$$

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Now, if you consider rows five and six in that equation and take a look in this equation. You can show that what they are telling as is the average kinetic energy in the first system minus plus average potential energy in the first system is given by this. Similarly, this for the second system; this is the equation. Now, if for instance, if coupling spring is absent, then we know that in the steady state average kinetic energy, same as average potential energy and that again this consistent.




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For  $k_c \ll k_1, k_2$  (light coupling/weak coupling)

$$\langle \dot{y}_1 \dot{y}_2 \rangle = \frac{k_c^2 \left[ \frac{c_1 + c_2}{m_1 + m_2} \right] \left[ \frac{m_1}{c_1} \langle F_1 \dot{y}_1 \rangle - \frac{m_2}{c_2} \langle F_2 \dot{y}_2 \rangle \right]}{\left[ \frac{k_1 + k_c}{m_1} - \frac{k_2 + k_c}{m_2} \right]^2 + \left[ \frac{c_1 + c_2}{m_1 + m_2} \right] \left[ \frac{c_1 (k_2 + k_c) + c_2 (k_1 + k_c)}{m_1 m_2} \right]}$$

$$\Rightarrow \langle \pi_{12} \rangle = h_{12} \left[ \frac{m_1}{c_1} \langle \pi_{1,m} \rangle - \frac{m_2}{c_2} \langle \pi_{2,m} \rangle \right]$$

$$h_{12} = \frac{k_c^2 \left[ \frac{c_1 + c_2}{m_1 + m_2} \right]}{\left[ \frac{k_1 + k_c}{m_1} - \frac{k_2 + k_c}{m_2} \right]^2 + \left[ \frac{c_1 + c_2}{m_1 + m_2} \right] \left[ \frac{c_1 (k_2 + k_c) + c_2 (k_1 + k_c)}{m_1 m_2} \right]}$$


Now, if we assume that this coupling spring is much smaller than  $k_1$  or  $k_2$ , that means the coupling is weak. This is one of the assumption that is made in statistical energy analysis, and if I now consider expected value of  $y_1$  into  $y_2$  dot, you can derive this expression and you have to carefully look at this expression. This is expected value of  $y_1$  into  $y_2$  dot is nothing but expected value of  $\pi_{12}$  multiply for the  $k_c$ . We denote this multiplier.

See, the term inside this bracket is of importance. You can see that this is  $\pi_1$  divided by  $c_1$  into expected value of  $\pi_1$  in and is  $m_2$  by  $c_2$  into  $\pi_2$  in, the second term. So, what this is telling is that the energy flow from 1 to 2 is proportional to the, difference in energy at difference in the power input between the first and the second system some scaled factor. Now, we will manipulate this and see what it tells us.

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For  $k_c \ll k_1, k_2$  (light coupling/weak coupling)

$$\langle \pi_{1,in} \rangle = \langle \pi_{1,diss} \rangle = c_1 \langle \dot{y}_1^2 \rangle = \frac{2c_1}{m_1} \langle KE_1 \rangle$$

$$\langle \pi_{2,in} \rangle = \langle \pi_{2,diss} \rangle = c_2 \langle \dot{y}_1^2 \rangle = \frac{2c_2}{m_2} \langle KE_2 \rangle$$

$$\langle KE_1 \rangle = \langle PE_1 \rangle \quad ||$$

$$\langle KE_2 \rangle = \langle PE_2 \rangle \quad ||$$

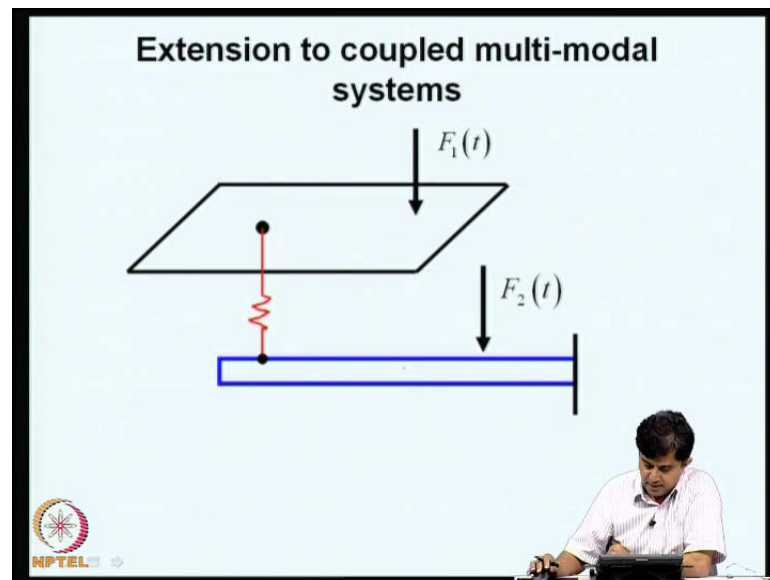
$\Rightarrow$  Fundamental SEA result

$$\langle \pi_{12} \rangle = h_{12} [\langle KE_1 + PE_1 \rangle - \langle KE_2 + PE_2 \rangle]$$

$h_{12}$  = coupling loss factor

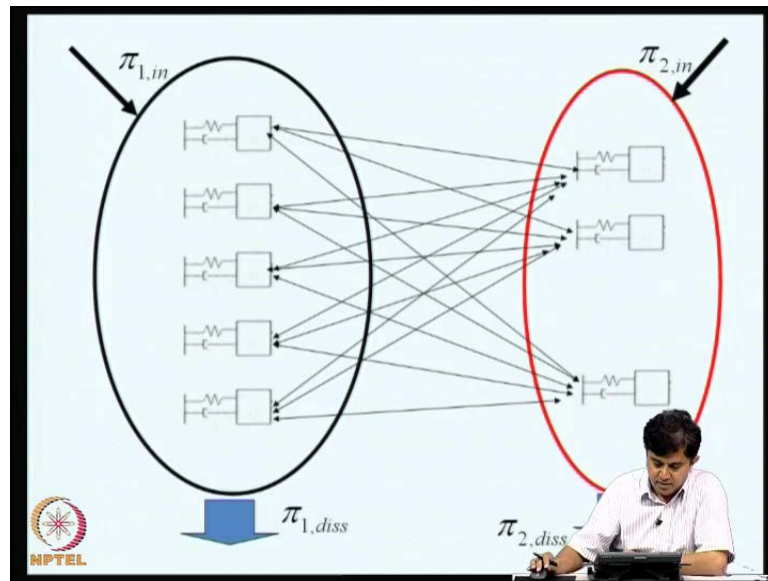
Now, again under the assumption of light coupling, we can show that the power input is in steady state is dissipated by the system. So, that is  $c_1 \dot{y}_1^2$ . So, I can write for  $\pi_{1,in}$  mean  $2c_1/m_1$  into  $KE_1$ . Similarly, expected value of  $\pi_{2,in}$  can be given by this expression, and we use the fact that in steady state, average kinetic energy and average potential energy for the two systems are equal, and if I now substitute, I get what is known as fundamental statistical energy analysis result namely that, average energy flow is proportional to the difference between, the energy between two subsystems, the energy in the two subsystem; that means just like thermal energy flows from hot spot to cold spot, the vibration energy also flows from places of high energy concentration to low energy concentration.

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So, this is the essential fundamental result of SEA and this is, this has basis in random vibration principles. That is what I am trying to discuss. Only that aspect of SEA is what I am discussing. We have consider two oscillators. Now, what happens if there are two continuous systems? So, for example, plate is coupled to a cantilever beam. This plate itself is a multi-modal subsystem. It has several natural of frequencies in its uncoupled state. If I were to perform a model analysis in the frequency range of interest, there may be several modes; that means this plate will be represented by a set of single degree freedom systems corresponding to the normal modes in the independent, generalized coordinates of this plate. Similarly, the cantilever beam also can be represented as set of oscillators.

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Now, that we notionally represent like this. This first picture here is, this is a plate and these are the oscillators that correspond to generalize coordinates in the frequency range of interest. Similarly, this is a beam where there are few more oscillators here which corresponds to the generalize coordinates of the cantilever beam. This system receives some power  $\pi_1$  in and it dissipate some power  $\pi_1$  diss. Similarly, this system receives  $\pi_2$  in and dissipates  $\pi_2$  in. Now, we have studied how energy exchange takes place between two oscillators. Now, what we do is we assume that the energy flow from plate to cantilever consists of energy exchange between the oscillators in this set with oscillators in this set; that means this oscillator exchanges energy with this, this and this. Similarly, this oscillator exchanges energy with all the oscillators here. So, that means all these oscillators exchange energies.

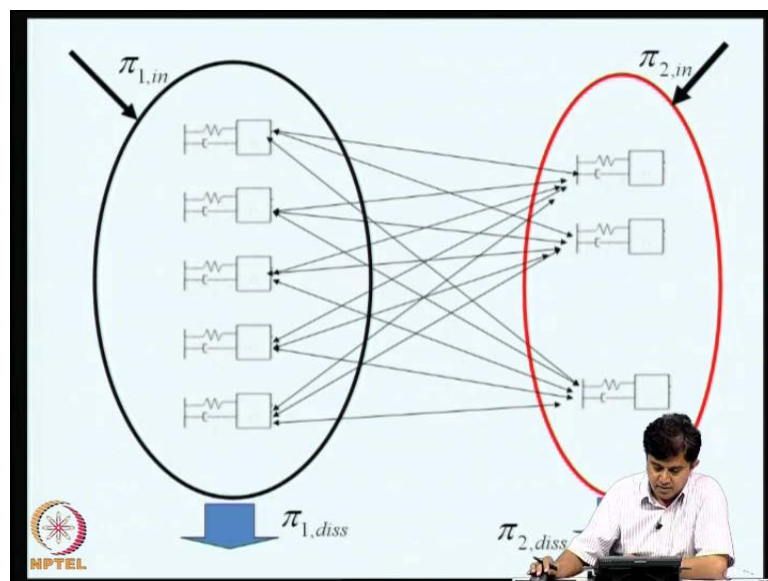
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$$\langle \pi_{\alpha\beta} \rangle = \omega \left[ \frac{1}{\Delta\omega} \sum_{i=1}^{N_\alpha} \sum_{j=1}^{N_\beta} n_{ij} \right] \left\{ \frac{\langle E_\alpha \rangle}{n_\alpha} - \frac{\langle E_\beta \rangle}{n_\beta} \right\}$$

$\omega$  = central frequency  
 $\Delta\omega$  = frequency bandwidth  
 $\langle E_\alpha \rangle$  = total average energy in subsystem  $\alpha$   
 $n_\alpha$  = number of modes per unit frequency interval for the subsystem  $\alpha$ .

Energy flow between conservatively coupled linear subsystems excited by broad band random excitation is proportional to the difference between subsystem average modal energies

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Now, the average power flow from alpha to beta - that means plate to the cantilever - is given by the, a constant multiplier which is again dependent on the plate and cantilever characteristics and in some sense in an energy difference between the two plates, that is, total energy divided by number of oscillator that are participated. What is an energy density or the concentration of energy is total energy divided by number of oscillator? It is like temperature in thermal analogy. So, it is a hot means total energy divided by number of modes is high. So, here, total energy divided by number of modes here. So, the flow takes place from hot spot to the cold spot and that the energy flow is

proportional to the energy difference and its proportionality constant involves the properties of the structure.

So, we have  $\omega$  is a central frequency;  $\Delta\omega$  is a frequency bandwidth and  $e_\alpha$  is total average energy in the subsystem  $\alpha$  and  $n_\alpha$  is a number of modes per unit frequency interval for the subsystem  $\alpha$ ; that means  $\omega$ , that defines this that the way energy is defined this, this, factor is relevant. So, what this means is - energy flow between conservatively coupled linear subsystems excited by a broad band random excitation is proportional to the difference subsystem average model energies. Now, this proportionality constant etcetera - this is known as coupling loss matrix, coupling loss factor matrix - the elements all that have to be determined. By, in this particular case, when we discuss two oscillators, we derived this h 1 2. That is shown here using random vibration principles.

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**SEA equation**

$$\omega \begin{bmatrix} (\eta_i + \eta_{ij}) & -\eta_{ji} \\ -\eta_{ij} & (\eta_j + \eta_{ji}) \end{bmatrix} \begin{Bmatrix} E_i \\ E_j \end{Bmatrix} = \begin{Bmatrix} \Pi_{mi} \\ \Pi_{mj} \end{Bmatrix}$$

Matrix of coupling loss factors      Unknowns      Input power

So, the S e a equation, the equation that you will actually solve when you apply statistical energy analysis will be in this form. The unknowns are these energy levels  $E_i$  and  $E_j$ . This is  $E_1$  and  $E_2$  in this particular 2 subsystem problem, and the excitations or the inputs in terms of power  $\pi_i$  in 1  $\pi_j$  in j. This is known; these are unknowns, and this is analogs to are stiffness matrix but this is known as matrix of coupling loss factors. Typically, this coupling loss factors could be estimated experimentally where you know the difficulty is associated with joint flexibilities, I mean randomness etcetera the system

properties can be taken in to account or analytically using wave propagation concepts. I will not get in to that.

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**SEA equation for a system of  $n$ -subsystems**

$$\omega \begin{bmatrix} \left( \eta_1 + \sum_{i=1}^{N_1} \eta_{1i} \right) & -\eta_{12} & \dots & -\eta_{1N_s} \\ -\eta_{21} & \left( \eta_2 + \sum_{i=1}^{N_2} \eta_{2i} \right) & \dots & -\eta_{2N_s} \\ \vdots & \vdots & \ddots & \vdots \\ -\eta_{N_s 1} & -\eta_{N_s 2} & \dots & \left( \eta_{N_s} + \sum_{i=1}^{N_s} \eta_{N_s i} \right) \end{bmatrix} \begin{Bmatrix} E_1 \\ E_2 \\ \vdots \\ E_{N_s} \end{Bmatrix} = \begin{Bmatrix} \Pi_{in1} \\ \Pi_{in2} \\ \vdots \\ \Pi_{inN_s} \end{Bmatrix}$$

The same logic if you applied to a set of  $n$  subsystems, we get the unknowns are  $E_1, E_2, E_3, \dots, E_n$  - where  $n$  is the number of subsystems.  $\Pi_{in1}, \Pi_{in2}, \dots, \Pi_{in n}$  are the inputs, and this is the matrix of coupling loss factors which has to be determined as a experimentally or by analytically, and the principles of random vibration analysis are relevant in this context.

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**Further reading**

- F J Fahy, 1994, Statistical energy analysis : a critical review, Philosophical Transactions of Royal Society of London, A346, 431– 447.
- R.S. Langley, A general derivation of the statistical energy analysis equations for coupled dynamic systems, Journal of Sound and Vibration 135 (1989) 499-508
- A J Keane, 1992, Energy flow between arbitrary configurations of conservatively coupled multi-modal elastic subsystems. Proceedings of Royal Society of London, A, 436, 537-568.

So, this subject of statistical energy analysis quiet worst and I have given few of you suggestions for further reading. There is a review paper by Fahy's statistical energy analysis a critical review, and the two papers - one by Langley and another by Keane which contain quiet useful information. Now, with this, we will conclude this present lecture. In the next set of four lectures, we will consider certain problems which basically would help you to learn the subject. So, the discussion on principles of random vibration analysis and the methods and the basic problems etcetera now closes in the remaining four lectures of this course. We will consider a set of problems and tackle them. We conclude this lecture at this stage.